

Molalla Rd Apartments

Traffic Impact Analysis

Woodburn, Oregon

Date:

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CHAPTER 1: INTRODUCTION AND SUMMARY

This study evaluates the transportation impacts for the proposed Molalla Road Apartments residential development in Woodburn, Oregon. Currently, the parcel of land is a greenfield and adjacent to commercial and industrial uses. The proposed development would construct 231 apartment units. The development would have a single point of access to Molalla Road.

The purpose of this transportation impact analysis (TIA) is to evaluate possible system impacts from the proposed development and, where necessary, recommend mitigation measures on the nearby transportation network. The impact analysis is focused on intersections identified as being in the study area, based on guidance from City and ODOT staff, and shown in **Figure 1**.

1. N Pacific Hwy (99E) / Molalla Road (OR 211)
2. Molalla Road (OR 211) / June Way
3. Molalla Road (OR 211) / Proposed Site Driveway
4. Molalla Road (OR 211) / Cooley Road

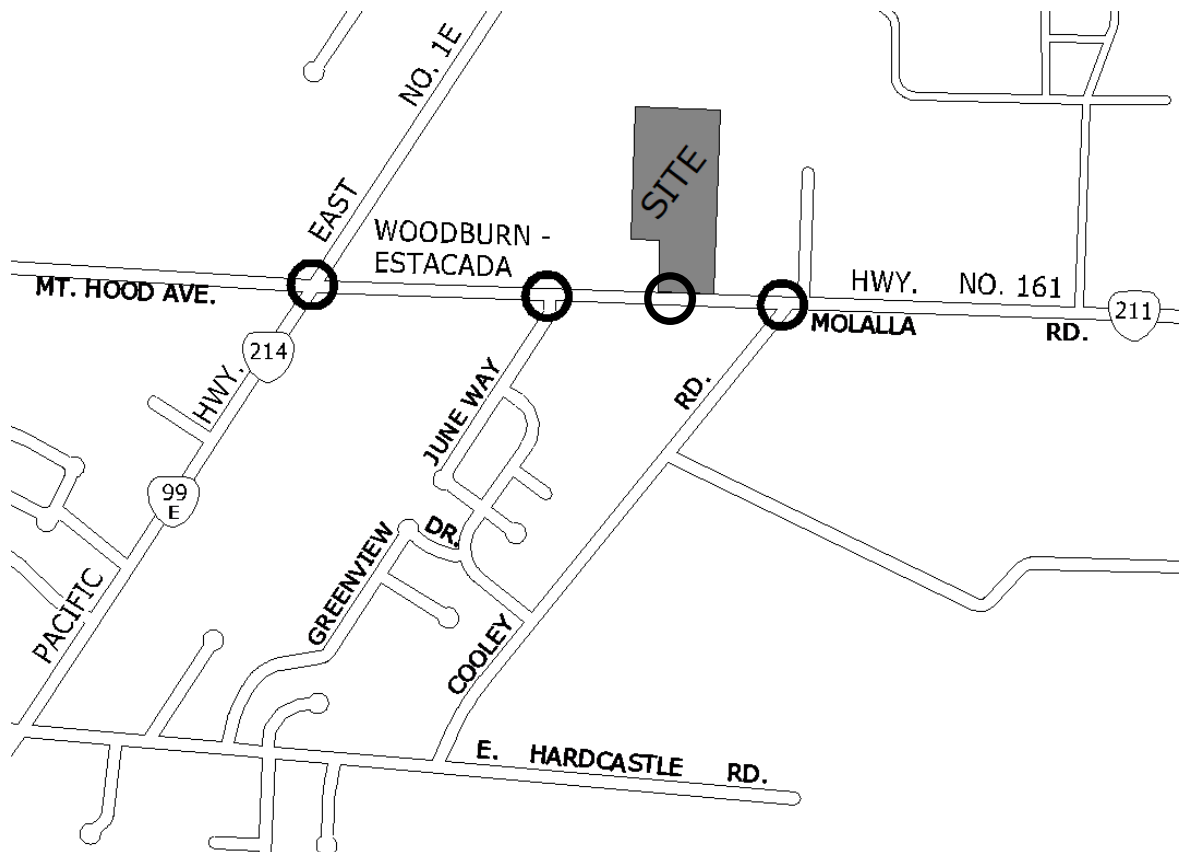


Figure 1: Study Area



Proposed Development

The project sponsor is proposing to build an apartment complex located on the north side of Molalla Road just east of N Pacific Highway (Hwy) at 2145 Molalla Road. The proposed development would build two-hundred thirty-one (231) apartment units. The complex would have direct access to Molalla Road. **Appendix A** provides the site plan of the proposed development. **Table 1** lists important characteristics of the study area and proposed project.

Table 1: Key Study Area and Proposed Development Characteristics

Characteristics	Information
Study Area	
Number of Study Intersections	Two
Analysis Period	Weekday A.M and P.M. Peak Hours
Analysis Scenarios	2019 Existing Conditions, AM Peak Hour 2019 Existing Conditions, PM Peak Hour 2021 Background Traffic, AM Peak Hour 2021 Background Traffic, PM Peak Hour 2021 Total Traffic (Background + Site), AM Peak Hour 2021 Total Traffic (Background + Site), PM Peak Hour
Project Site	
Existing Land Use	Vacant
Proposed Development	231 apartment units
Project Access	The development will be served via a single access to Molalla Road



Existing Conditions and Intersection Operations

Transportation impacts associated with the proposed development in the nearby area were evaluated. The following intersections were identified for evaluation:

1. N Pacific Hwy (99E) / Molalla Road (OR 211)
2. Molalla Road (OR 211) / June Way
3. Molalla Road (OR 211) / Proposed Site Driveway
4. Molalla Road (OR 211) / Cooley Road

Table 2 shows the existing intersection operations at the study intersections.

Table 2: Existing Traffic at Study Intersection Operations

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.77 V/C	<i>0.94 V/C</i>
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.17 V/C	0.01 V/C, 0.16 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	-	-	-	-
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.19 V/C	0.14 V/C, 0.32 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement

LOS = Level of Service of Worst Movement

Locations exceeding mobility standards are shown with ***bold/italicized***

Project Traffic Impact

Construction of the proposed Molalla Road apartments is expected to be completed in 2021. To determine whether the proposed project will result in off-site traffic impacts, future traffic volumes were estimated. **Tables 3 and 4** provide the intersection operations for the future scenarios with and without project traffic.

Table 3: 2021 Background Intersection Operations (Without Project)

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.80 V/C	0.97 V/C
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.18 V/C	0.01 V/C, 0.17 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	-	-	-	-
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.19 V/C	0.15 V/C, 0.33 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement

LOS = Level of Service of Worst Movement

Locations exceeding mobility standards are shown with **bold/italicized**

Table 4: 2021 Total Intersection Operations (With Project)

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.84 V/C	1.00 V/C
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.20 V/C	0.01 V/C, 0.20 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.02 V/C, 0.10 V/C	0.06 V/C, 0.07 V/C
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.20 V/C	0.15 V/C, 0.37 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement

LOS = Level of Service of Worst Movements

Locations exceeding mobility standards are shown with **bold/italicized**



Key Findings

Key findings associated with the proposed development include the following items:

- The proposed development would generate 78 (20 in, 58 out) AM peak hour trips and 99 (61 in, 38 out) PM peak hour vehicle trips.
- All study intersections are expected to operate within mobility standards with the addition of the proposed site, with the exception of N Pacific Hwy (99E) / Molalla Road (OR 211). This location exceeds the target mobility standard during the PM Peak hour under the existing, background, and total (with project) analysis scenarios.

CHAPTER 2: EXISTING CONDITIONS

This chapter provides documentation of existing study area conditions, including the project site, study area roadway network, and existing traffic volumes and operations.

Project Site

This study evaluates the transportation impacts for the proposed Molalla Road apartments in Woodburn, Oregon. Currently, the parcel of land is a greenfield and adjacent to other housing. The proposed development would construct 231 apartment units. The development would have a single point of access to Molalla Road.

Project Area Roadway Network

Key roadways in the study area are summarized in **Table 5** along with their existing roadway characteristics.

Table 5: Project Area Existing Roadway Characteristics

Roadway	Classification	Ownership	Number of Lanes	Posted Speed	Pedestrian Facilities	Bicycle Facilities
N Pacific Hwy (99E)	Minor Arterial (north of OR 211), Principal Arterial (south of OR 211)	ODOT	4	35 mph	Sidewalks on both sides of the street	Bike lanes on both sides of the street
Molalla Road (OR 211)	Minor Arterial (east of 99E), Principal Arterial (west of 99E)	ODOT	2 (east of 99E), 4 (west of 99E)	25 mph (east of June Way) 35 mph (west of June Way)	Sidewalks on both sides of the street west of 99E. Intermittent sidewalks on the south side between 99E and Cooley Rd	Shoulder greater than 5' on the south side. Shoulder greater than 5' on the north side between June Way and Cooley Rd
June Way	Local	City	2	25 mph	Sidewalks on both sides	None
Cooley Road	Major Collector	City	2	40 mph	None	None



Existing Traffic Volumes and Operations

Existing AM and PM peak hour traffic operations were analyzed at the following study intersections:

1. N Pacific Hwy (99E) / Molalla Road (OR 211)
2. Molalla Road (OR 211) / June Way
3. Molalla Road (OR 211) / Proposed Site Driveway
4. Molalla Road (OR 211) / Cooley Road

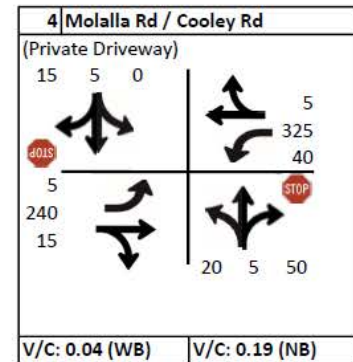
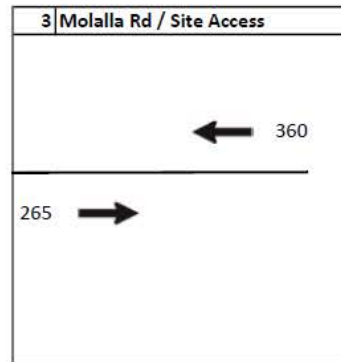
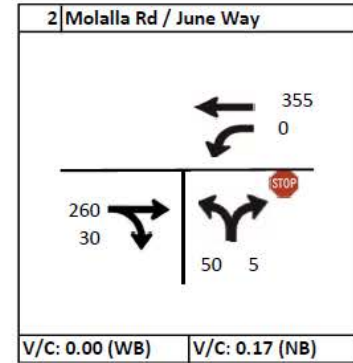
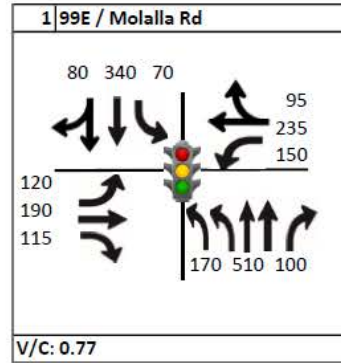
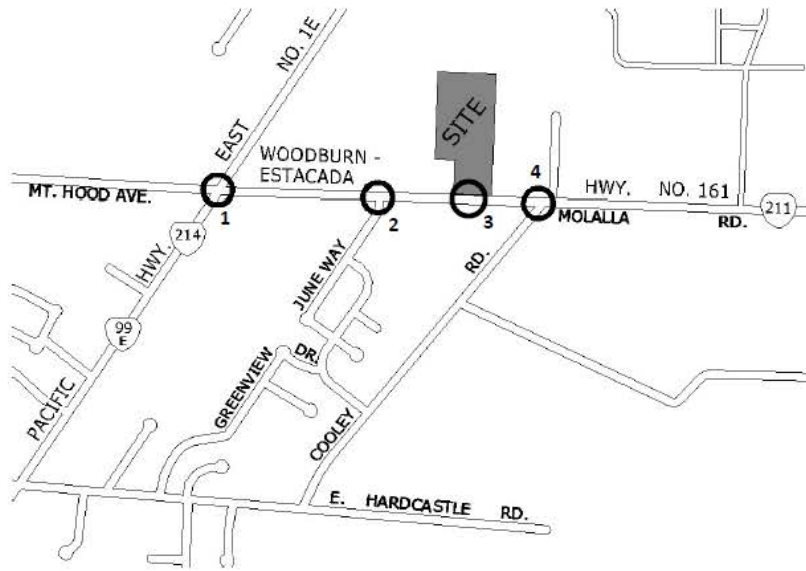
Peak hour traffic counts were collected on Thursday, September 12th, 2019. The peak hour traffic volumes analyzed under existing conditions are shown in **Figure 2 and Figure 3**, with the detailed two-hour traffic counts included in **Appendix B**. The AM system peak hour was identified as 7:05 – 8:05 AM, and the PM system peak hour as 4:10 – 5:10 PM.

ODOT has requested that a minimum of one vehicle be represented for each permitted movement in the analysis to ensure the software used (Synchro and SimTraffic) accurately calculate the resulting operational values. The field collected data did not record any vehicles for the following movements:

- Westbound left at Molalla Road / June Way (AM Peak Hour – Existing and Background Conditions Only)
- Southbound left at Molalla Road / Cooley Road (AM Peak Hour)
- Westbound right at Molalla Road / Cooley Road (PM Peak Hour)
- Northbound thru at Molalla Road / Cooley Road (PM Peak Hour)

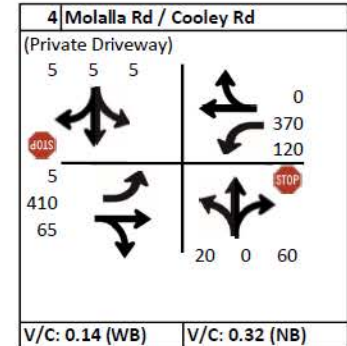
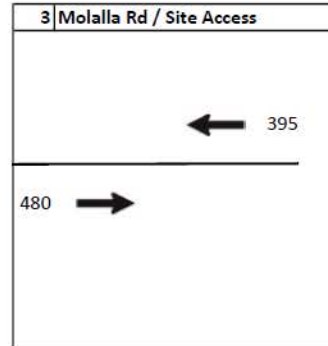
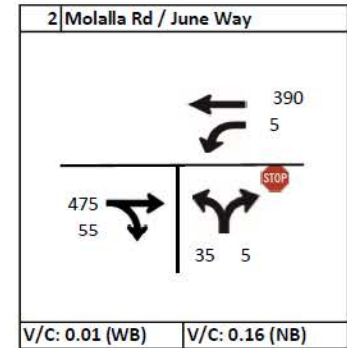
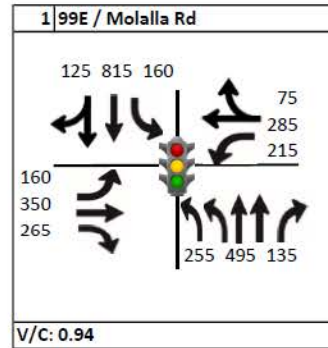
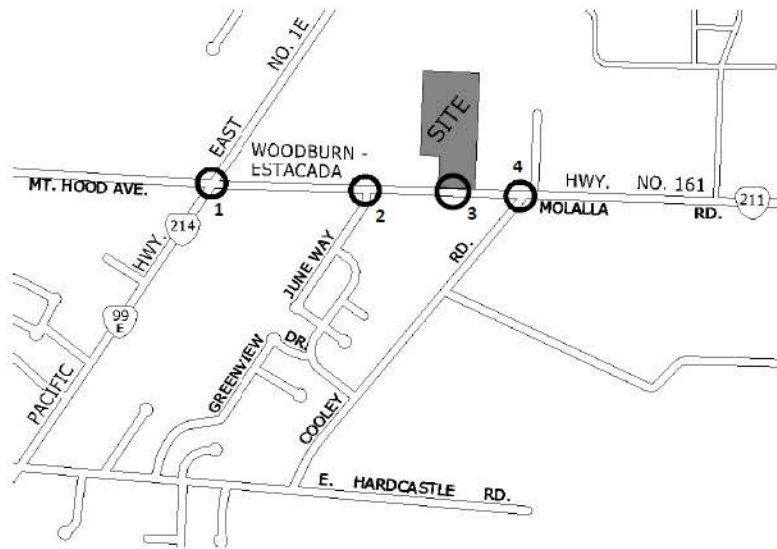
One vehicle for each of these movements has been added to the analysis models for all scenarios and is reflected in the reports in the appendix. Since these values were not field observed, they are not shown in the Figures throughout the report. Additionally, all analysis scenarios assume a saturation flow rate of 1750 pcphg, as requested by ODOT.

Figure 2: 2019 AM Peak Hour Existing Conditions



Note: Volumes shown represent 30th Highest Volume Values rounded up to nearest increment of 5.

Figure 3: 2019 PM Peak Hour Existing Conditions



Note: Volumes shown represent 30th Highest Volume Values rounded up to nearest increment of 5.

Existing Operating Conditions

Existing traffic operations at the study intersections were evaluated for the AM and PM peak hours using 30th highest hour volumes based on the Oregon DOT Analysis Procedure’s Manual methodology. The estimated V/C ratio of each study intersection is shown in **Table 6** and is based on the 2000 Highway Capacity Manual¹ methodology for signalized intersections and 2016 Highway Capacity Manual methodology² for unsignalized intersections. **Appendix C** provides detailed reports summarizing these results. **Appendix D** provides information on how 30th highest hour volumes were developed for analysis.

All study intersections meet existing mobility standards, with the exception of N Pacific Hwy (99E)/ Molalla Road (OR 211) in the PM Peak Hour.

Table 6: 2019 Existing Intersection Operations

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.77 V/C	0.94 V/C
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.17 V/C	0.01 V/C, 0.16 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	-	-	-	-
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.19 V/C	0.14 V/C, 0.32 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement
 LOS = Level of Service of Worst Movement
 Locations exceeding mobility standards are shown with **bold/italicized**

Crash Analysis

The five most recent years of crash records (Jan 1, 2013- Dec 31, 2017) for the study area were obtained from Oregon Department of Transportation (ODOT’s) online database. A copy of these records is provided in **Appendix H**. Crashes recorded within approximately 250 feet of the study intersection are grouped with that intersection for analysis purposes.

During the five years evaluated, seventy-seven (77) crashes occurred at the study area intersections. The study intersection with the highest frequency of crashes over the five year period was N Pacific Hwy

¹ 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2000

² Highway Capacity Manual 6th Edition: A Guide for Multimodal Mobility Analysis, Transportation Research Board, Washington DC, 2016.



(99E) / Molalla Road (OR 211) with a total of seventy-two (72) crashes. The most common crash type at the N Pacific Hwy (99E) / Molalla Road (OR 211) intersection was rear-end collisions. There were no fatal crashes recorded at the study intersections, however there was one Injury A (incapacitating injury) crash recorded. There are three (3) recorded crashes at the intersection of Molalla Road (OR 211) / June Way and two (2) recorded at Molalla Road (OR 211) / Cooley Road.

Table 7: Number of Crashes at Study Intersections

Study Intersection	2013	2014	2015	2016	2017	Grand Total
N Pacific Hwy (99E) / Molalla Road (OR 211)	16	12	13	18	13	72
Molalla Road (OR 211) / June Way	1	-	1	1	-	3
Molalla Road (OR 211) / Cooley Road	2	-	-	-	-	2
Total	19	12	14	19	13	77

Table 8: Crash Type and Severity by Study Intersection (2013 - 2017)

Collision Type	N Pacific Hwy (99E) / Molalla Road (OR 211)	Molalla Road (OR 211) / June Way	Molalla Road (OR 211) / Cooley Road
Angle	2 (Injury C)	-	1 (Injury C)
Backing	2 (1 Injury C, 1 PDO)	-	-
Fixed Object or Other-object	3 (1 Injury A, 2 PDO)	-	1 (Injury B)
Head-On	2 (1 Injury B, 1 PDO)	-	-
Non-Collision	1 (PDO)	-	-
Pedestrian	3 (2 Injury B, 1 Injury C)	-	-
Rear-End	49 (2 Injury B, 23 Injury C, 24 PDO)	2 (Injury C)	-
Sideswipe-Overtaking	3 (PDO)	-	-
Turning Movement	8 (1 Injury B, 2 Injury C, 4 PDO)	1 (Injury B)	-
Total	72	3	2

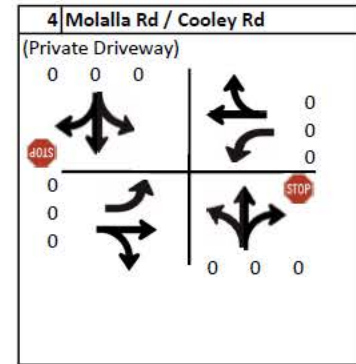
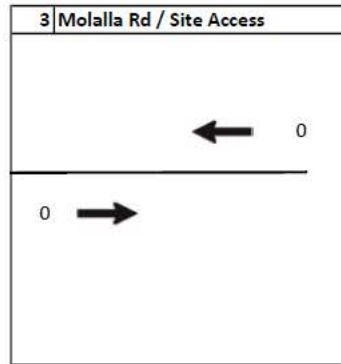
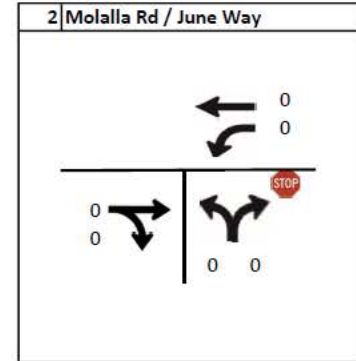
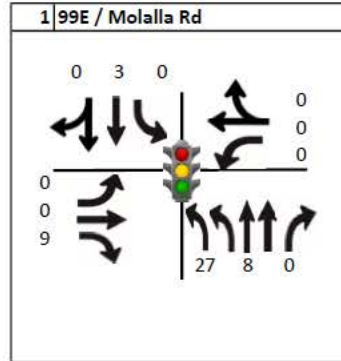
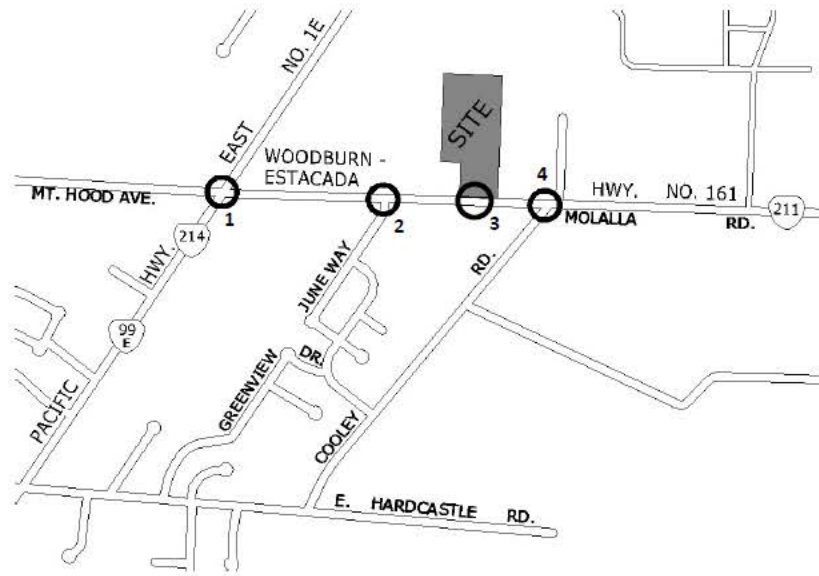
Note: PDO = Property Damage Only crash

CHAPTER 3: BACKGROUND TRAFFIC

Construction of the proposed Molalla Road apartments is expected to be completed in 2021. To account for traffic growth from 2019 to 2021, a 1.4% growth rate was used to forecast the future background traffic volumes on roads within the study area. Growth rate calculations are provided in **Appendix D**.

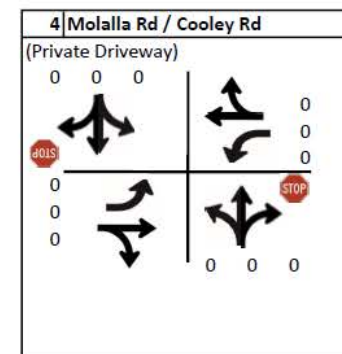
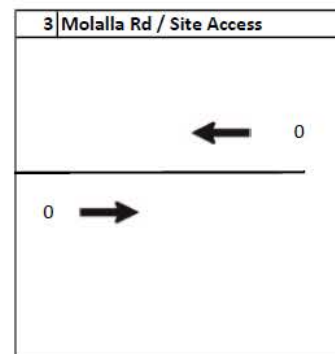
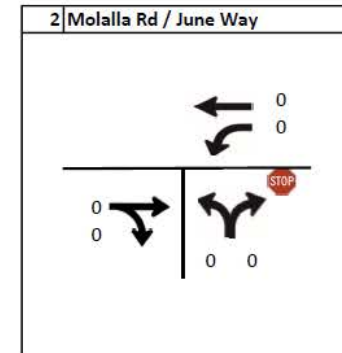
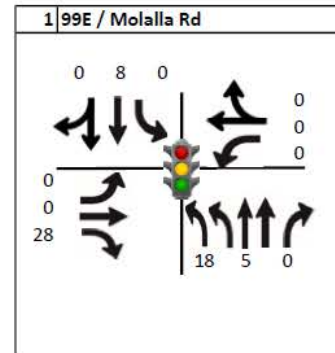
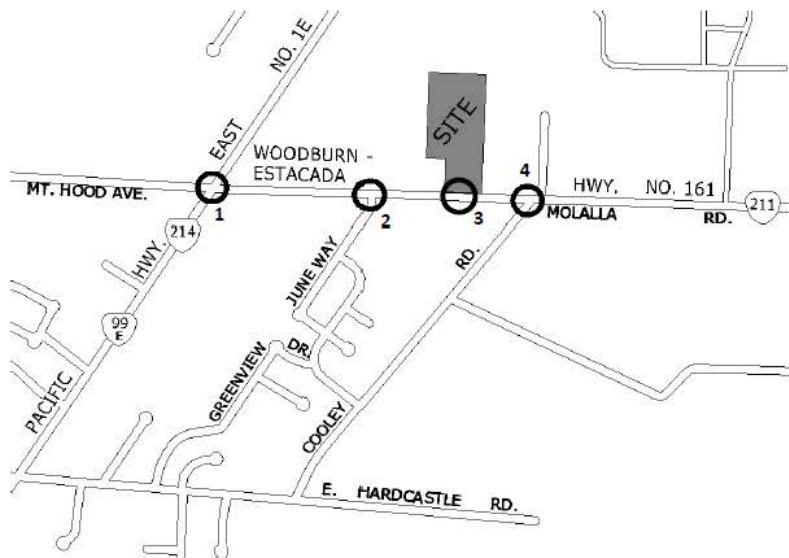
The City of Woodburn identified that the Pacific Valley Apartments should be treated as “in-process”, with the developments trips included in the background and total traffic volumes. The Pacific Valley Apartments study identified trips traveling through one of the study intersections for the Molalla Road Apartments; North Pacific Highway (OR99E)/Molalla Road. These in-process trips are identified in **Figures 4 and 5**. Documentation for the Pacific Valley Apartments is included in **Appendix E**. Background traffic volumes are show in **Figures 6 and 7**.

Figure 4: AM In-Process Development Trips



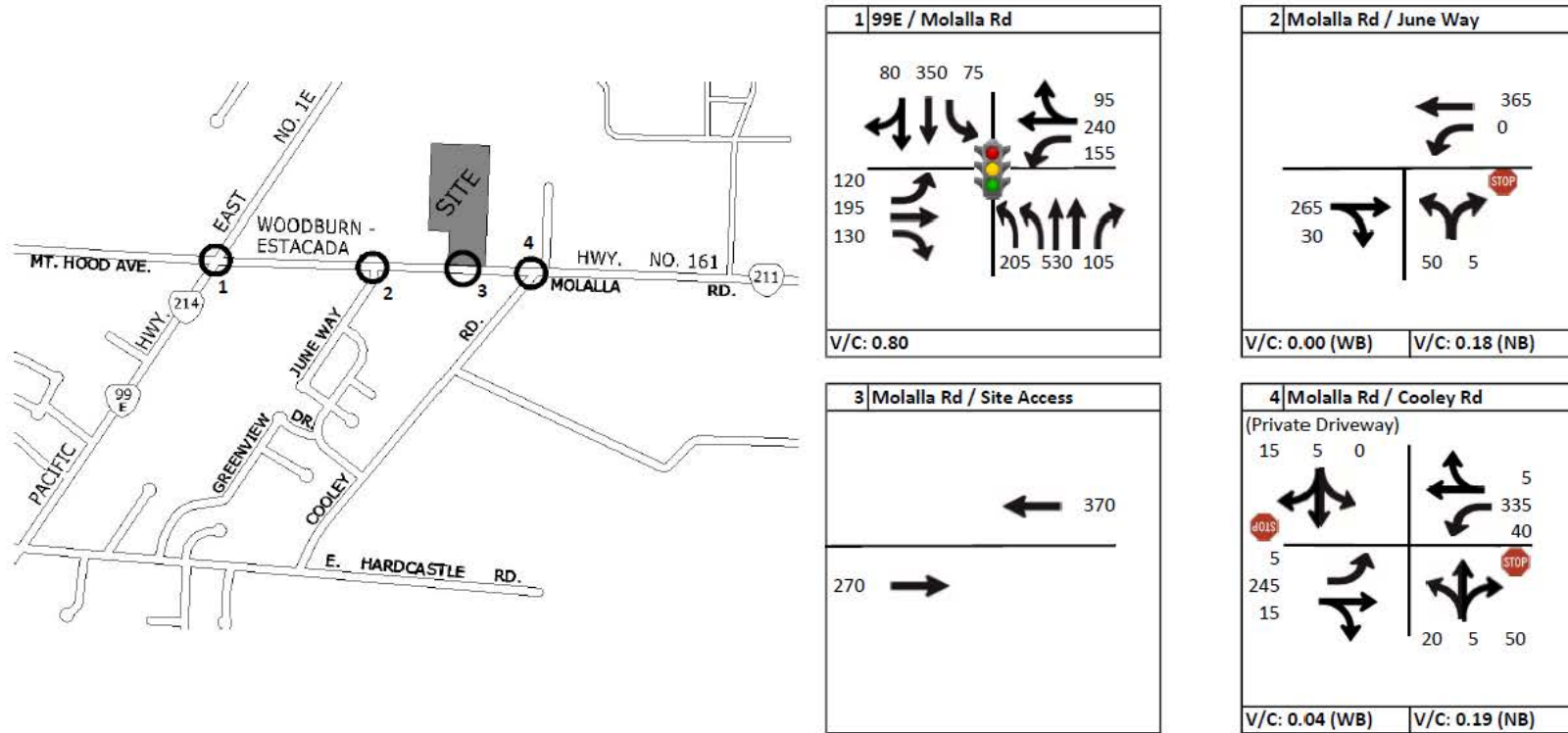
Note: Volumes shown are based on the Woodburn Apartments TIA, dated May 6th, 2019, by Engineer of Record Todd Mobley, PE

Figure 5: PM In-Process Development Trips



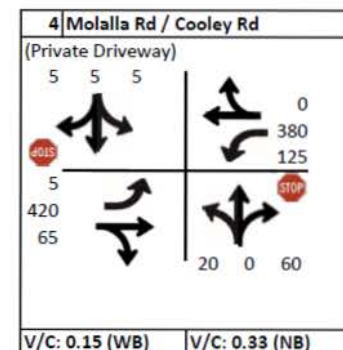
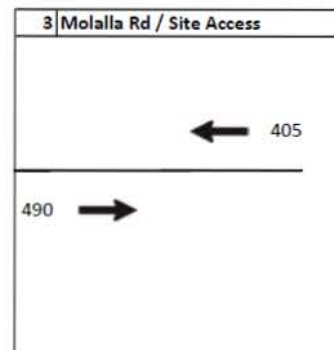
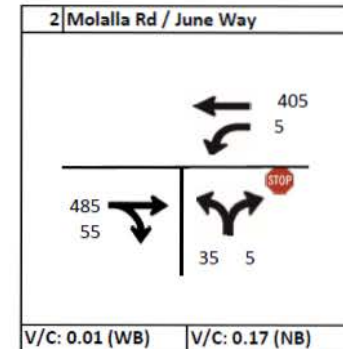
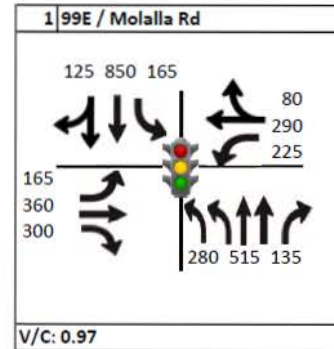
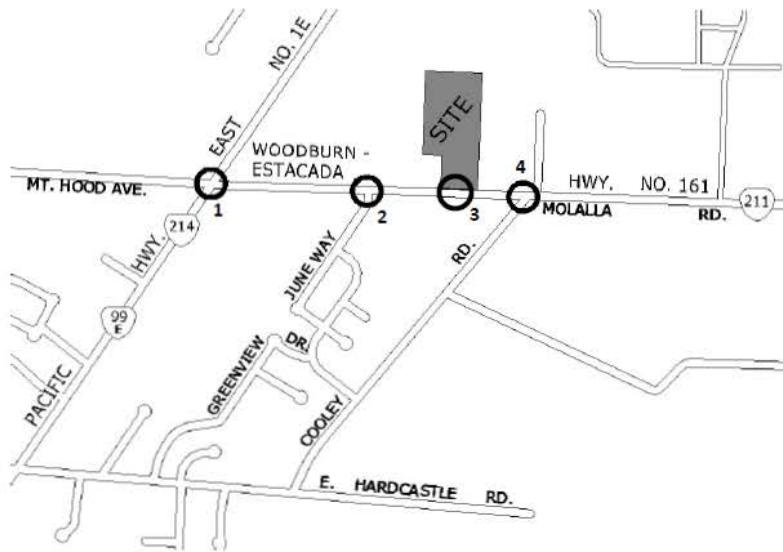
Note: Volumes shown are based on the Woodburn Apartments TIA, dated May 6th, 2019, by Engineer of Record Todd Mobley, PE

Figure 6: 2021 AM Peak Hour Background Conditions



Note: Volumes shown are rounded up to nearest increment of 5.

Figure 7: 2021 PM Peak Hour Background Conditions



Note: Volumes shown are rounded up to nearest increment of 5.



Background Intersection Operations

Background traffic operations at the study intersections were determined based on the 2016 Highway Capacity Manual methodology³ for unsignalized intersections and the 2000 Highway Capacity Manual methodology for signalized intersections⁴. The estimated V/C ratio of each study intersection is shown in **Table 9. Appendix F** provides detailed reports summarizing these results.

Table 9 lists intersection operations after accounting for the assumed traffic volume growth. All study intersections meet mobility standards, with the exception of N Pacific Hwy (99E)/ Molalla Road (OR 211) in the PM Peak Hour.

Table 9: 2021 Background Intersection Operations

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.80 V/C	<i>0.97 V/C</i>
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.18 V/C	0.01 V/C, 0.17 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	-	-	-	-
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.19 V/C	0.15 V/C, 0.33 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement

LOS = Level of Service of Worst Movement

Locations exceeding mobility standards are shown with ***bold/italicized***

³ Highway Capacity Manual 6th Edition: A Guide for Multimodal Mobility Analysis, Transportation Research Board, Washington DC, 2016.

⁴ 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2010.

CHAPTER 4: PROJECT IMPACTS

This chapter reviews the impacts that the proposed 231 unit apartment complex would have on the study area transportation system. The focus of the impact analysis is on the following study intersection:

1. N Pacific Hwy (99E) / Molalla Road (OR 211)
2. Molalla Road (OR 211) / June Way
3. Molalla Road (OR 211) / Proposed Site Driveway
4. Molalla Road (OR 211) / Cooley Road

Trip Generation

Trip generation is used to estimate the number of vehicle trips added to the roadway network by a development during a specified period. In this case, the AM and PM peak hour periods are studied. Trip generation estimates are established data and methodology provided by the Institute of Transportation Engineers (ITE).⁵ **Table 10** lists the peak hour vehicle trip generation estimates for this development.

Table 10: Molalla Road Apartments Trip Generation Summary

Land Use (ITE Codes)	Dwelling Units	Time Period	Trip Generation Rate	Peak Hour Trips		
				In	Out	Total
Multi-Family Mid-Rise (221)	231	AM Peak	Equation	20	58	78
TOTAL AM PEAK HOUR				20	58	78
Multi-Family Mid-Rise (221)	231	PM Peak	Equation	61	38	99
TOTAL PM PEAK HOUR				61	38	99

⁵ *Trip Generation, 9th Edition*, Institute of Transportation Engineers, 2012.



Trip Distribution

Trip distribution provides an estimation of where trips from the development originate and end on the study area network. This is represented as percentages where large portions of the trips generated enter and exit the project study area. Trip distribution patterns are calculated based on a combination of existing count data and recommendations from the City and ODOT. The trip distribution percentages are shown in **Figure 8** and resulting site distributed trips are summarized in **Figure 9 and Figure 10**.

Figure 8: Trip Distribution

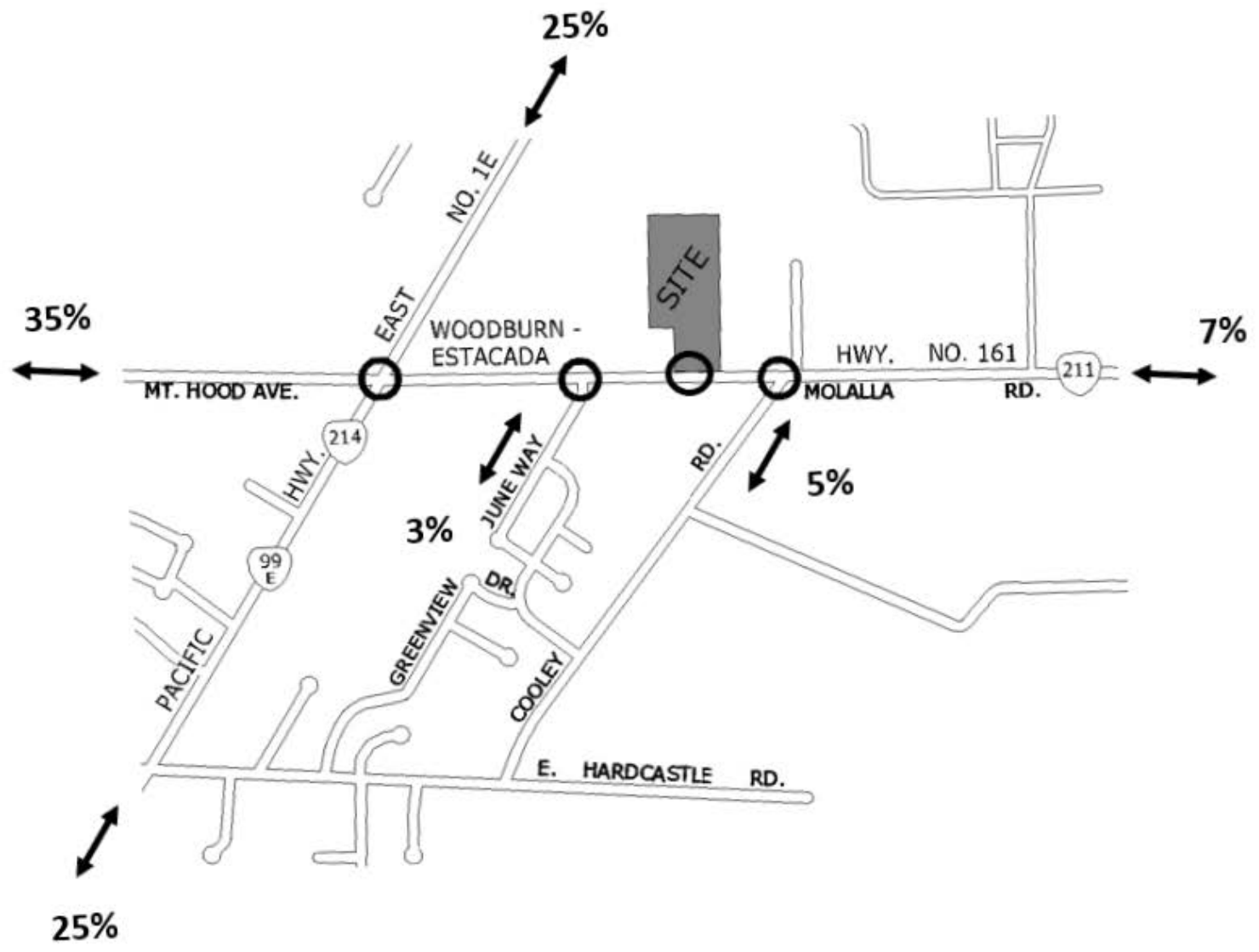


Figure 9: AM Peak Hour Site Generated Volumes

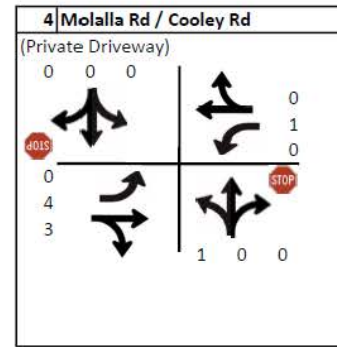
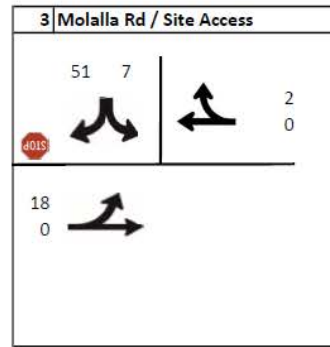
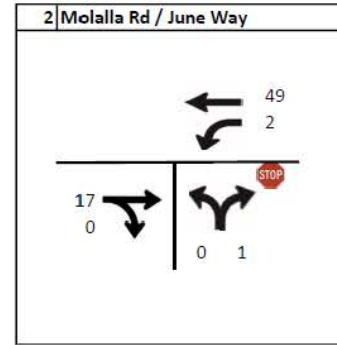
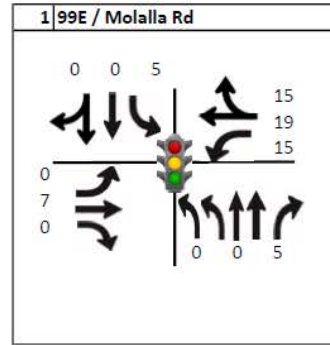
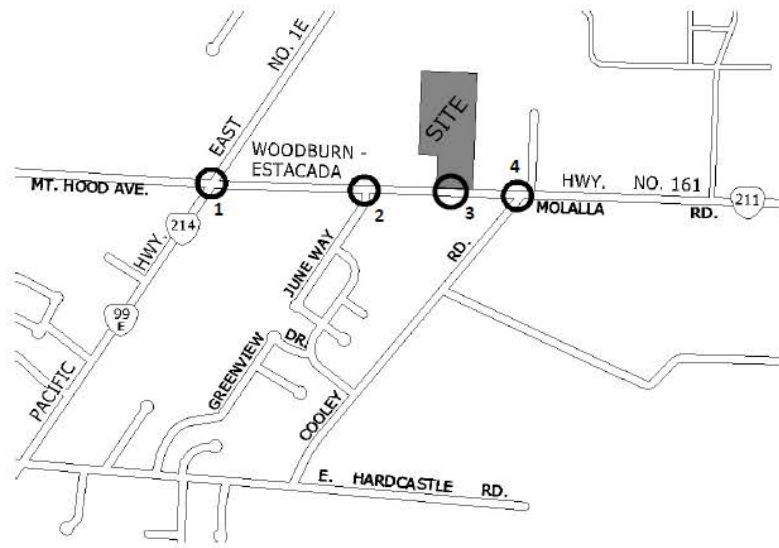
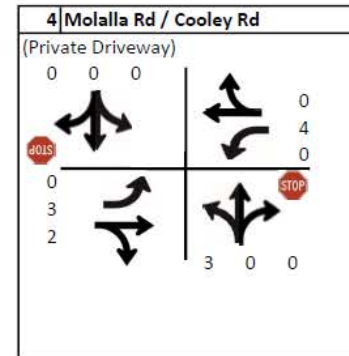
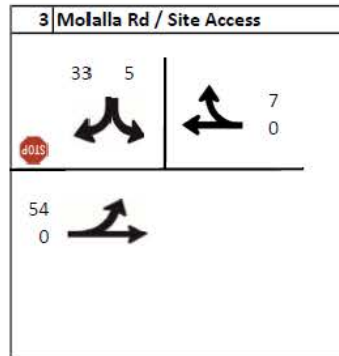
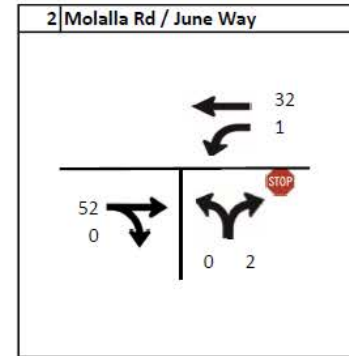
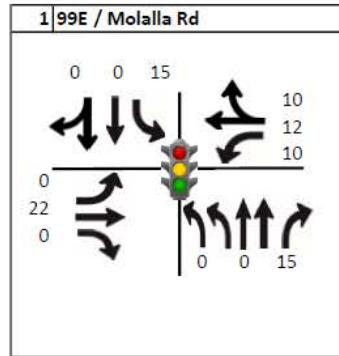
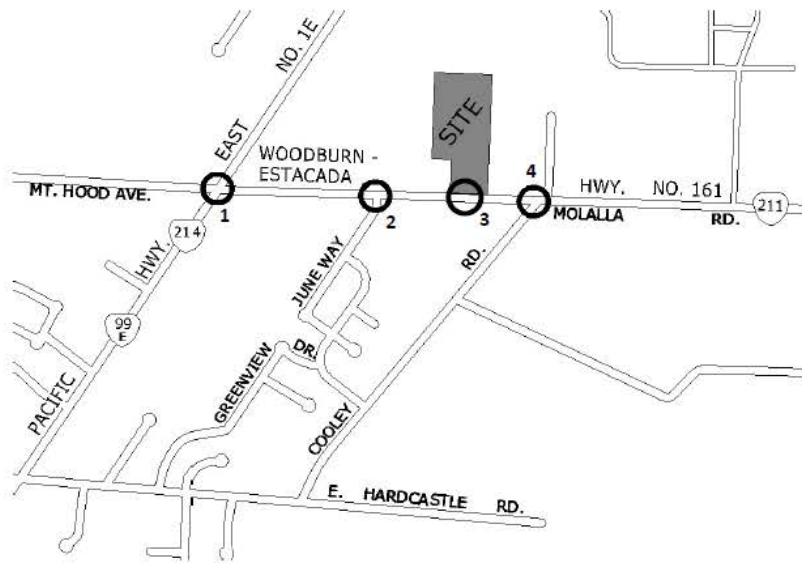


Figure 10: PM Peak Hour Site Generated Volumes

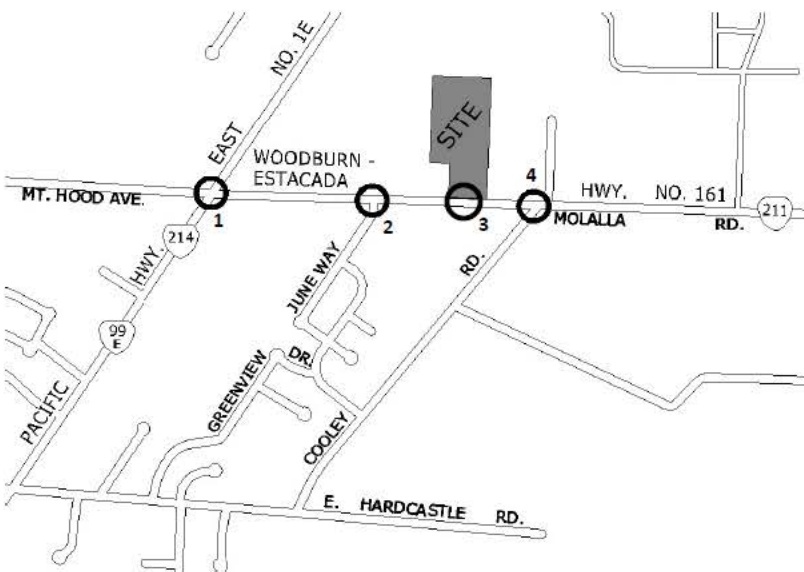




Future Traffic Volumes with the Proposed Development

The estimated trips associated with the proposed development were added to the background volumes to estimate the total traffic scenario traffic volumes. **Figure 11 and Figure 12** show the 2021 total traffic volumes used for analysis.

Figure 11: 2021 AM Peak Hour Total Traffic Volumes



1 99E / Molalla Rd	
<p>80 350 80</p>	<p>110 259 170</p>
<p>120 202 130</p>	<p>205 530 110</p>
V/C: 0.84	

2 Molalla Rd / June Way	
<p>414 2</p>	<p>STOP</p>
<p>282 30</p>	<p>50 6</p>
V/C: 0.00 (WB)	V/C: 0.20 (NB)

3 Molalla Rd / Site Access	
<p>51 7</p>	<p>2 370</p>
<p>18 270</p>	<p>STOP</p>
V/C: 0.02 (EB)	V/C: 0.10 (SB)

4 Molalla Rd / Cooley Rd	
<p>(Private Driveway)</p> <p>15 5 0</p>	<p>5 336 40</p>
<p>5 249 18</p>	<p>STOP</p> <p>21 5 50</p>
V/C: 0.04 (WB)	V/C: 0.20 (NB)

Figure 12: 2021 PM Peak Hour Total Traffic Conditions

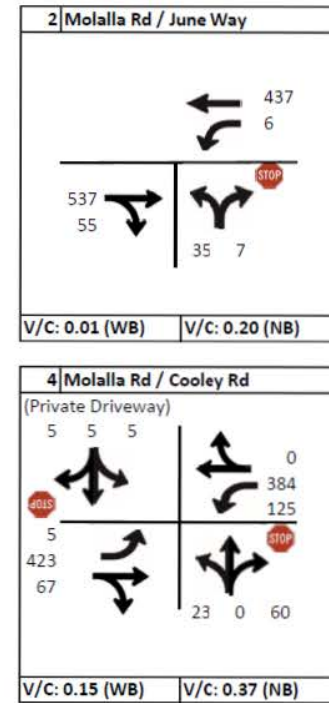
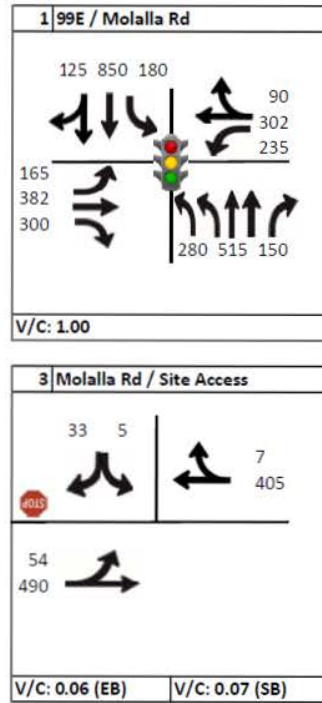
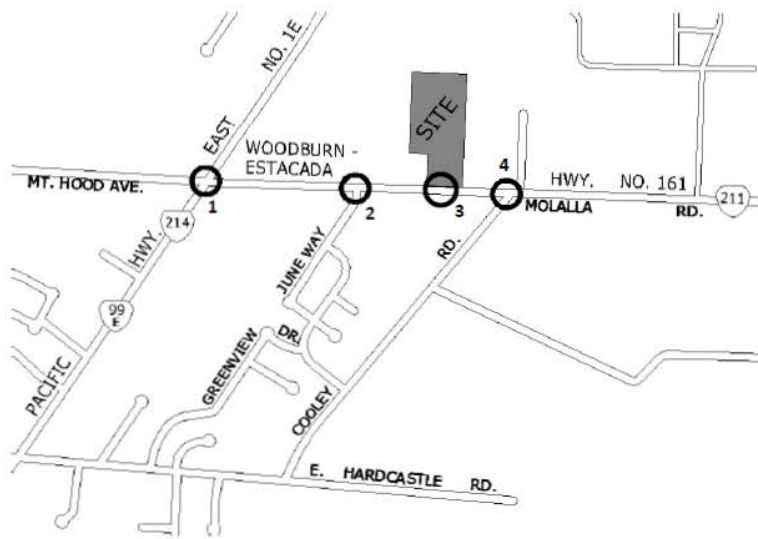




Table 11 lists the study intersection total traffic operating conditions for the AM and PM peak hours. Traffic operations at the study intersections were determined for the peak hours based on the 2016 Highway Capacity Manual methodology⁶ for unsignalized intersections and the 2000 Highway Capacity Manual methodology for signalized intersections⁷. The estimated V/C ratio of each study intersection is shown in **Table 11**. **Appendix F** provides detailed reports for the operational results.

Based on the operational analysis, all study intersections will function within their applicable mobility standards with the exception of N Pacific Hwy (OR 99E) at Molalla Road (OR 211), which exceeds target mobility standards during the P.M. Peak Hour scenario.

Table 11: 2021 Total Intersection Operations (with Project)

No.	Intersection	Traffic Control	Operating Standard (Major, Minor)	AM Peak Hour (Major, Minor)	P.M. Peak Hour (Major, Minor)
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	Signal	0.90 V/C	0.84 V/C	1.00 V/C
2	Molalla Road (OR 211) / June Way	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.00 V/C, 0.20 V/C	0.01 V/C, 0.20 V/C
3	Molalla Road (OR 211) / Proposed Site Driveway	Unsignalized (Two way stop)	0.95 V/C, 1.0 V/C	0.02 V/C, 0.10 V/C	0.06 V/C, 0.07 V/C
4	Molalla Road (OR 211) / Cooley Road	Unsignalized (Two way stop)	0.95 V/C, 0.95 V/C	0.04 V/C, 0.20 V/C	0.15 V/C, 0.37 V/C

V/C = Volume-to-Capacity Ratio of Worst Movement

LOS = Level of Service of Worst Movement

Locations exceeding mobility standards are shown with **bold/italicized**

Queuing Analysis

A queuing analysis is requested by ODOT for any intersection with a V/C that exceeds 0.70. **Table 12** provides queuing analysis for N Pacific Hwy (99E)/Molalla Road (OR 211) using SimTraffic calibrated per the ODOT Analysis and Procedures Manual Version 1. Both the northbound lefts and westbound lefts exceed available storage, however, these movements do not serve trips to or from the proposed site.

⁶ Highway Capacity Manual 6th Edition: A Guide for Multimodal Mobility Analysis, Transportation Research Board, Washington DC, 2016.

⁷ 2000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2010.

Table 12: N Pacific Hwy (99E) / Molalla Road (OR 211) Queuing Analysis

Movement	Available Storage	95 th Queue Length (ft)					
		2019 Existing Conditions		2021 Background Conditions		2021 Total Conditions	
		AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour	AM Peak Hour	PM Peak Hour
NB Left	350	380	370	375	385	375	370
NB Thru	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000
NB Right	230	<20	<20	<20	<20	<20	<20
SB Left	>1,000	420	410	430	400	410	425
SB Thru/Right	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000
EB Left	900	375	445	430	410	435	480
EB Thru	>1,000	865	880	895	850	850	850
EB Right	900	<20	235	235	215	130	255
WB Left	300	375	385	410	340	350	265
WB Thru/Right	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000	>1,000

Values are rounded up to the nearest increment of 5 feet.

Locations exceeding mobility standards are shown with ***bold/italicized***



Site Entrance Right-Turn Lane Evaluation

Molalla Road at the proposed site approach was evaluated for a dedicated right-turn lane using criteria from Section 12.2 of the ODOT Analysis Procedures Manual (AMP) Version 2. The provided criteria evaluates the need for a right turn lane based on volumes, crash experience, and special cases. A right-turn lane is not recommended based on the following criteria.

Vehicular Volumes

The vehicular volume criteria looks at the intersecting volume of the roadway and the proposed driveway. The PM Peak hour represents the highest use case, with seven westbound right turning vehicles entering the site (as opposed to 2 in the AM peak hour). The design hourly volume for the westbound approach (including right turn movements) is 412 for the PM Peak hour. Using the Exhibit 12-2 from the ODOT APM, the location does not meet warrants for a dedicated right turn lane at the entrance.

Crash Experience

The five most recent years of crash records (Jan 1, 2013- Dec 31, 2017) for the study area were obtained from Oregon Department of Transportation (ODOT's) online database. A copy of these records is provided in **Appendix H**.

Two crashes are recorded along the frontage of the proposed site. An Injury A, head-on collision was recorded on June 19th, 2014, at 5:24 PM. The notes from the crash record indicate the driver was operating the vehicle while drowsy/fatigued/asleep and driving with a revoked/suspended license. The driver of the vehicle departed from their travel lane, resulting in the crash. This crash appears to be the result of driver behavior/error.

A second crash was recorded September 15th, 2016, at 4:24 PM, and is documented as a property damage only, fixed/other-object crash. No additional details are provided in the crash data base.

Right-turn lanes can be used to mitigate rear-end crashes. No rear-end crashes are recorded along the frontage of the property. A right-turn lane is not recommended based on crash experience.

Special Cases

The proposed access is not located near any passing lanes or railroad crossings. The proposed access is located on a straight horizontal alignment of roadway. It is not currently recommended that a right turn lane be installed based on special cases criteria.



Key Findings

The key traffic impact findings associated with the proposed development include:

- The proposed development would generate 78 (20 in, 58 out) AM peak hour trips and 99 (61 in, 38 out) PM peak hour vehicle trips.
- All study intersections are expected to operate within mobility standards with the addition of the proposed site, with the exception of N Pacific Hwy (99E) / Molalla Road (OR 211). This location exceeds the target mobility standard during the PM Peak hour under the existing, background, and total (with project) analysis scenarios.



APPENDIX A: PROPOSED SITE PLAN

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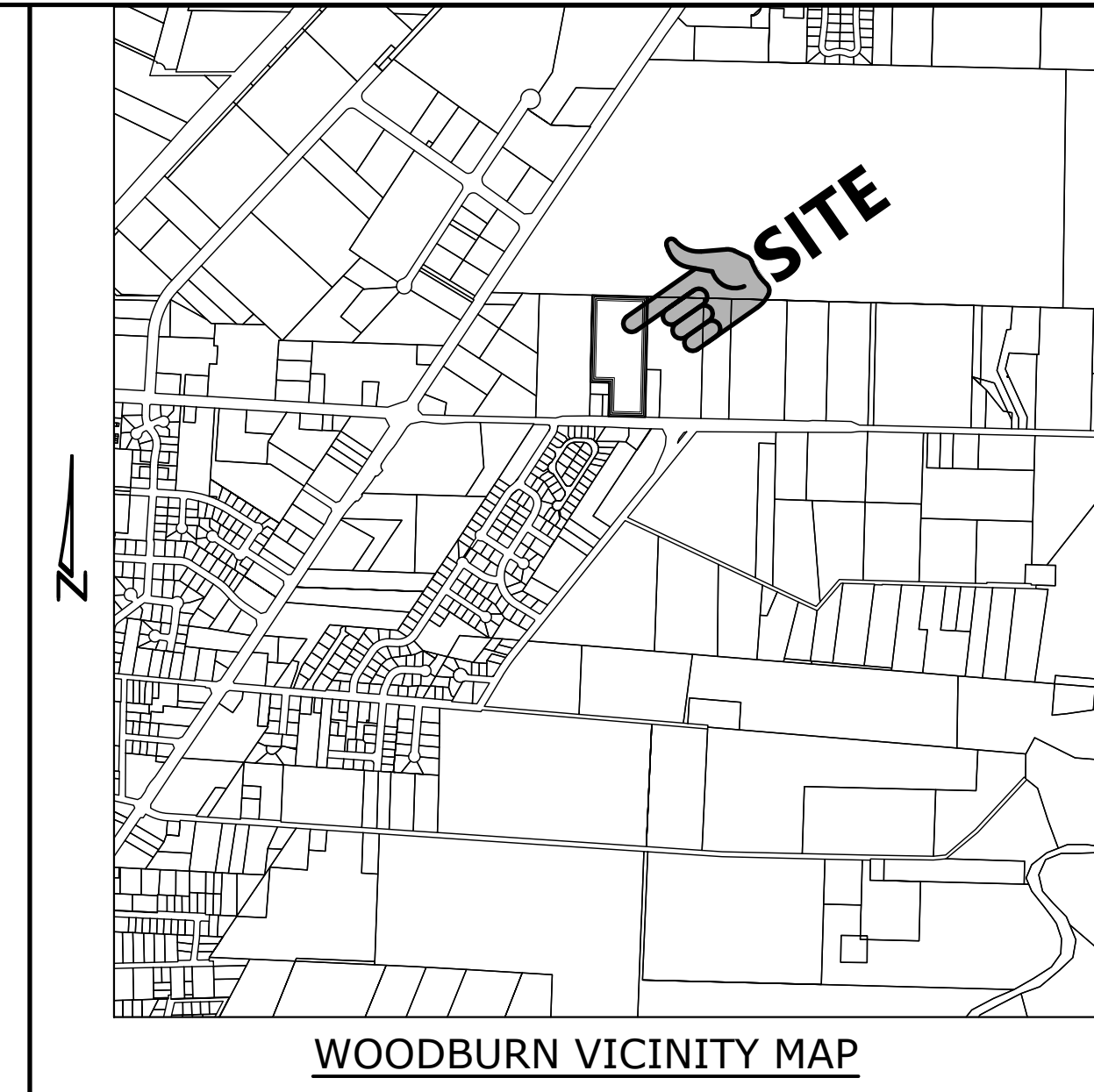
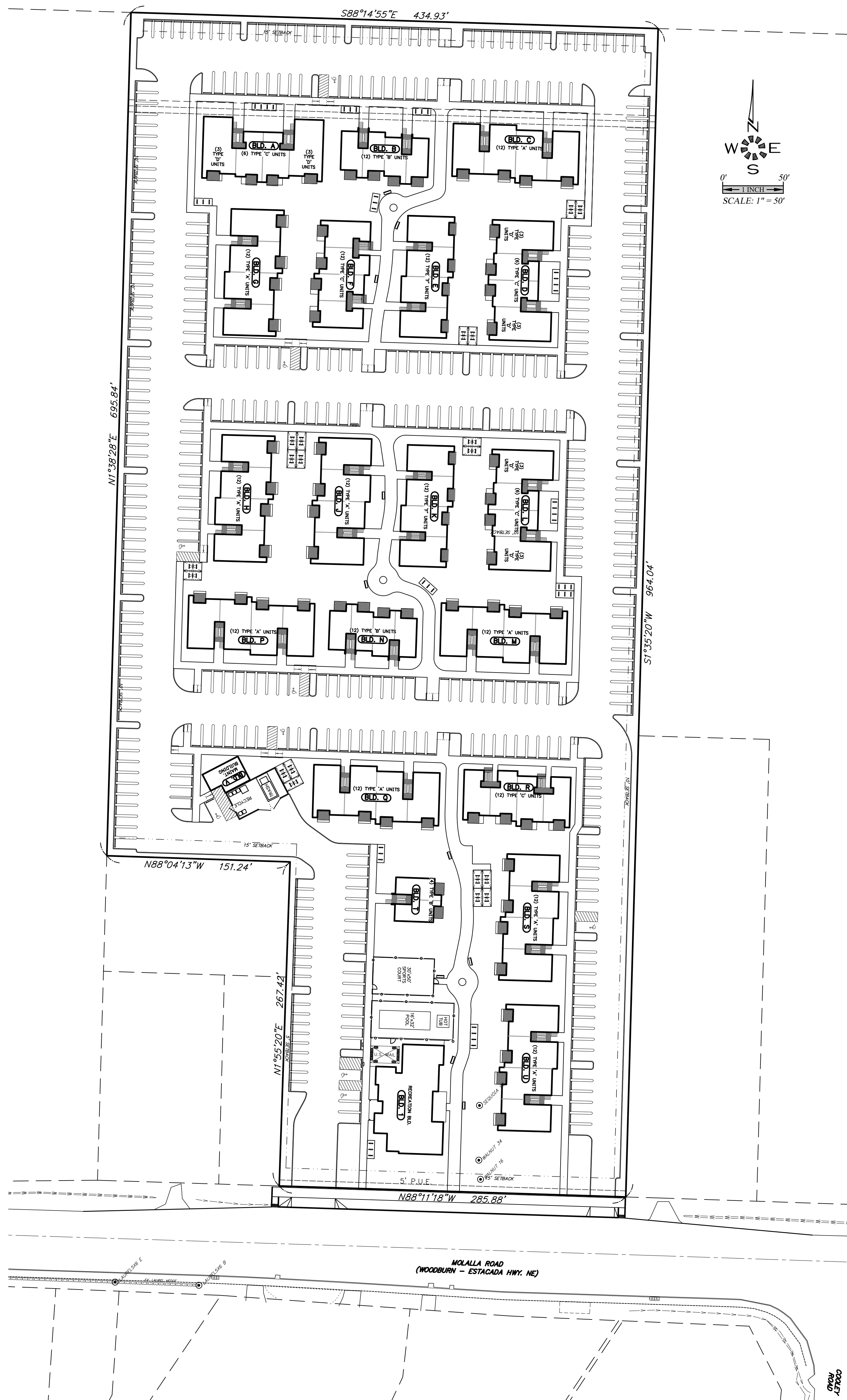
IVAN AND ELENA SMERIKOV

22091 300th ST. SE
ERSKINE, MN 56535
PHONE: (503) 655-7933

Owner / Developer:

I AND E CONST.

9550 SE CLACKAMAS RD.
CLACKAMAS, OREGON 97015
PHONE: (503) 655-7933

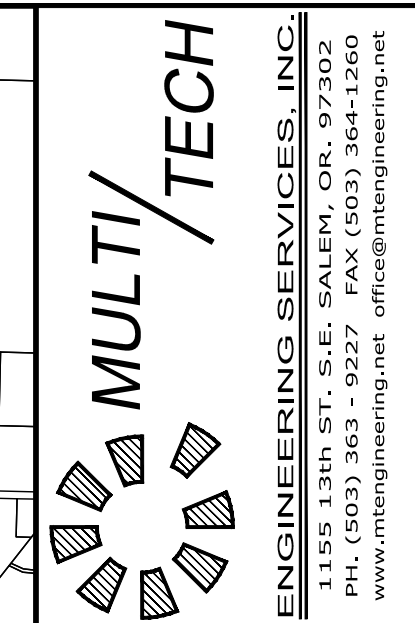


**WOODBURN
EASTSIDE
APARTMENTS**
SEC. 09, T. 5 S., R. 1 W., W.M.
CITY OF WOODBURN
MARION COUNTY, OREGON

SHEET INDEX

- SDR1 - COVER SHEET
- SDR2 - EXISTING CONDITIONS PLAN
- SDR3 - ANNEXATION PLAN
- SDR4 - PRELIMINARY SITE PLAN
- SDR5 - PRELIMINARY SITE PLAN
- SDR6 - SITE PLAN DETAILS
- SDR7 - PRELIMINARY OPEN SPACE PLAN
- SDR8 - PRELIMINARY PHOTOMETRIC PLAN
- SDR9 - PRELIMINARY DRAINAGE PLAN
- SDR10 - PRELIMINARY SANITARY SEWER PLAN
- SDR11 - PRELIMINARY DOMESTIC WATER AND FIRE SERVICE PLAN

- L1.1 SCHEMATIC LANDSCAPE PLAN
- L1.2 SCHEMATIC LANDSCAPE PLAN
- L1.3 LANDSCAPE NOTES & DETAILS

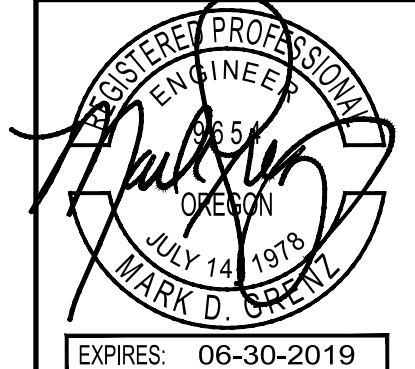


COVER SHEET

WOODBURN EASTSIDE APARTMENTS

NO CHANGES, MODIFICATIONS OR REPRODUCTIONS TO BE MADE TO THESE DRAWINGS WITHOUT WRITTEN AUTHORIZATION FROM THE DESIGN ENGINEER.
DIMENSIONS & NOTES TAKE PRECEDENCE OVER GRAPHICAL REPRESENTATION.

Design: M.D.G.
Drawn: C.D.S.
Checked: J.C.B.
Date: JUNE 2019
Scale: AS SHOWN



JOB # 6855

SDR1

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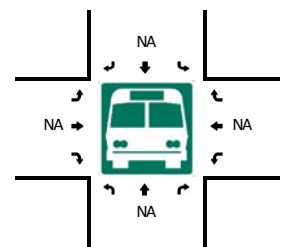
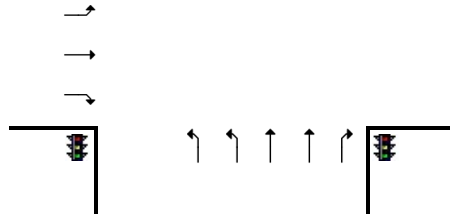
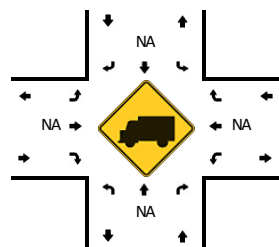
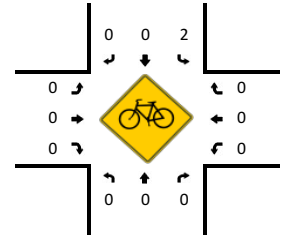
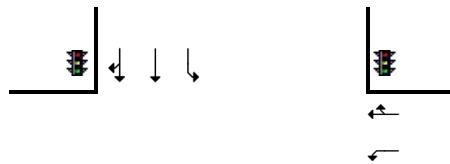
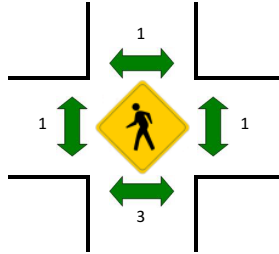
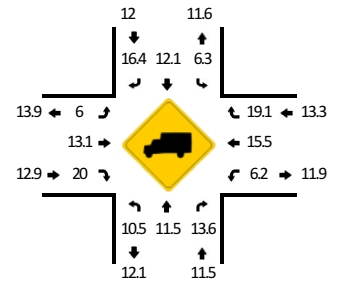
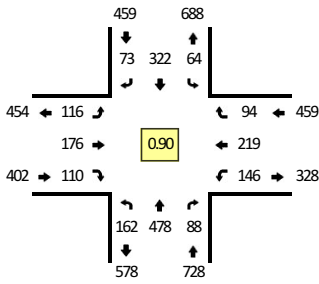


APPENDIX B: TRAFFIC COUNTS

LOCATION: OR-99E -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064701
DATE: Thu, Sep 12 2019

Peak-Hour: 7:10 AM -- 8:10 AM
Peak 15-Min: 7:45 AM -- 8:00 AM



5-Min Count Period Beginning At	OR-99E (Northbound)				OR-99E (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
6:00 AM	8	32	7	0	3	9	5	0	9	13	2	0	2	10	9	0	109	
6:05 AM	2	49	6	0	1	13	3	0	6	14	1	0	2	10	7	0	114	
6:10 AM	4	44	9	0	2	11	1	0	10	16	1	0	2	11	2	0	113	
6:15 AM	4	49	6	0	3	19	4	0	10	13	1	0	9	10	6	0	134	
6:20 AM	5	48	9	0	2	17	5	0	6	17	5	0	6	14	6	0	140	
6:25 AM	5	38	9	0	5	11	4	0	8	25	5	0	7	12	5	0	134	
6:30 AM	5	37	9	0	6	17	7	0	9	16	4	0	5	12	8	0	135	
6:35 AM	5	53	10	0	2	20	5	0	8	9	2	0	6	19	6	0	145	
6:40 AM	6	69	10	0	3	29	8	0	7	29	7	0	5	18	10	0	201	
6:45 AM	6	41	8	0	6	24	6	0	15	18	3	0	9	10	7	0	153	
6:50 AM	13	55	7	0	3	22	3	0	19	11	3	0	5	14	10	0	165	
6:55 AM	9	34	10	0	6	15	4	0	12	19	8	0	15	13	7	0	152	1695
7:00 AM	8	53	10	0	5	33	8	0	11	10	5	0	11	11	4	0	169	1755
7:05 AM	9	40	10	0	6	20	5	0	5	19	12	0	5	9	3	0	143	1784
7:10 AM	12	39	8	0	8	27	11	0	7	8	8	0	12	20	3	0	163	1834
7:15 AM	7	36	13	0	4	21	7	0	7	14	6	0	19	21	16	0	171	1871
7:20 AM	10	47	5	0	8	24	2	0	13	15	1	0	10	17	9	0	161	1892
7:25 AM	20	45	10	0	4	23	9	0	10	14	4	0	15	17	14	0	185	1943
7:30 AM	11	40	9	0	2	20	7	0	9	12	11	0	7	20	6	0	154	1962
7:35 AM	18	37	7	0	6	29	7	0	13	11	9	0	14	17	5	0	173	1990
7:40 AM	12	41	8	0	3	27	3	0	11	10	5	0	13	17	6	0	156	1945
7:45 AM	18	53	8	0	8	29	7	0	7	19	15	0	13	24	6	0	207	1999
7:50 AM	14	38	8	0	2	31	10	0	12	24	11	0	15	19	6	0	190	2024
7:55 AM	16	30	3	0	9	30	1	0	11	18	19	0	7	17	8	0	169	2041
8:00 AM	14	37	4	0	6	39	4	0	6	13	7	0	11	22	6	0	169	2041
8:05 AM	10	35	5	0	4	22	5	0	10	18	14	0	10	8	9	0	150	2048
8:10 AM	13	27	5	0	4	28	5	0	4	8	5	0	11	14	3	0	127	2012
8:15 AM	8	32	5	0	5	18	6	0	12	11	18	0	6	12	4	0	137	1978
8:20 AM	19	28	10	0	4	27	11	0	7	17	5	0	6	13	10	0	157	1974
8:25 AM	12	33	7	1	3	18	11	0	6	16	5	0	6	13	3	0	134	1923
8:30 AM	6	27	6	0	2	28	10	0	11	11	10	0	8	18	8	0	145	1914
8:35 AM	21	23	6	0	9	25	4	0	6	17	11	0	15	11	9	0	157	1898
8:40 AM	10	25	6	0	8	18	10	0	10	18	14	0	8	12	8	0	147	1889
8:45 AM	13	33	8	0	5	24	4	0	8	12	16	0	8	11	8	0	150	1832
8:50 AM	13	30	4	0	8	17	8	0	7	5	11	0	12	13	9	0	137	1779
8:55 AM	9	26	11	0	10	20	11	0	10	15	8	0	6	9	2	0	137	1747

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	192	484	76	0	76	360	72	0	120	244	180	0	140	240	80	0	2264
Heavy Trucks	32	60	20		0	24	8		8	12	28		12	36	4		244
Pedestrians		8				0				4				0			12
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Railroad																	
Stopped Buses																	

Comments:

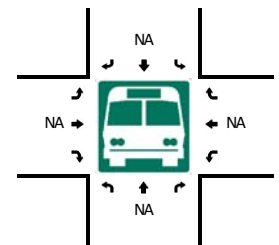
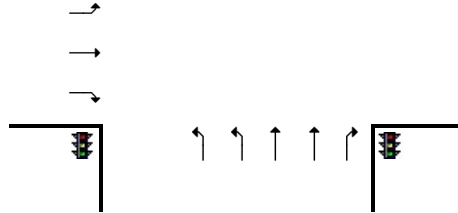
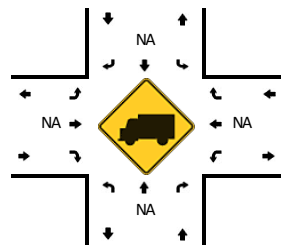
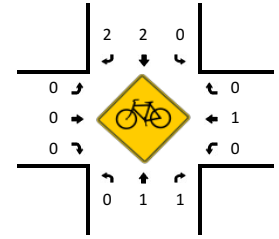
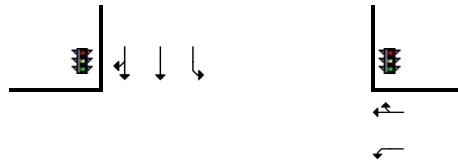
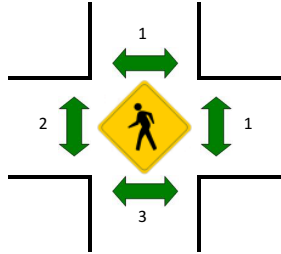
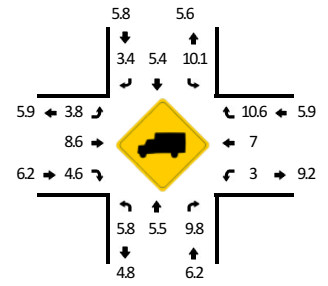
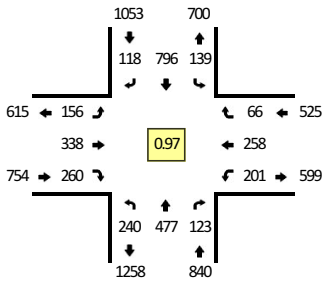
Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

LOCATION: OR-99E -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064702
DATE: Thu, Sep 12 2019

Peak-Hour: 4:25 PM -- 5:25 PM
Peak 15-Min: 4:55 PM -- 5:10 PM



5-Min Count Period Beginning At	OR-99E (Northbound)				OR-99E (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
3:00 PM	13	29	14	0	10	37	9	0	18	19	15	0	16	11	7	0	198	
3:05 PM	10	28	9	0	10	39	9	0	10	25	20	0	18	23	7	0	208	
3:10 PM	22	34	9	0	9	46	8	0	11	28	18	0	15	14	5	0	219	
3:15 PM	13	24	10	0	12	53	13	0	14	13	17	0	17	21	5	0	212	
3:20 PM	12	24	2	0	7	46	5	0	11	33	15	0	17	22	4	0	198	
3:25 PM	24	43	6	0	6	52	16	0	14	20	17	0	7	8	9	0	222	
3:30 PM	14	35	11	0	21	58	21	0	12	13	27	0	13	20	5	0	250	
3:35 PM	20	26	6	0	9	63	6	0	18	23	26	0	25	21	5	0	248	
3:40 PM	30	45	9	0	7	64	14	0	8	30	16	0	11	10	7	0	251	
3:45 PM	21	38	12	0	15	70	14	0	11	20	17	0	16	22	5	0	261	
3:50 PM	11	36	8	0	10	46	9	0	20	25	28	0	25	24	1	0	243	
3:55 PM	31	45	10	0	11	42	3	0	17	29	17	0	10	22	8	0	245	2755
4:00 PM	15	36	12	0	10	85	15	0	10	25	13	0	7	15	4	0	247	2804
4:05 PM	22	46	10	0	18	54	16	0	9	25	25	0	17	24	8	0	274	2870
4:10 PM	18	33	14	0	13	42	6	0	8	27	21	0	12	30	8	0	232	2883
4:15 PM	26	41	12	0	3	68	11	0	11	26	23	0	20	18	6	0	265	2936
4:20 PM	12	40	8	0	14	53	8	0	10	23	17	0	22	25	4	0	236	2974
4:25 PM	21	31	8	0	13	43	14	0	17	35	20	0	18	31	6	0	257	3009
4:30 PM	25	47	6	0	14	74	10	0	7	26	19	0	14	15	6	0	263	3022
4:35 PM	16	44	10	0	18	80	7	1	17	22	21	0	12	22	5	0	275	3049
4:40 PM	20	28	9	0	11	61	8	0	15	30	34	0	24	28	7	0	275	3073
4:45 PM	20	43	12	0	11	59	9	0	13	34	16	0	16	18	8	0	259	3071
4:50 PM	14	45	10	0	14	96	11	0	11	21	11	0	15	15	5	0	268	3096
4:55 PM	20	39	7	0	13	64	3	0	15	36	22	0	18	24	6	0	267	3118
5:00 PM	31	50	10	0	16	52	8	0	12	29	19	0	18	27	6	0	278	3149
5:05 PM	17	27	18	1	9	84	15	0	16	23	29	0	15	14	4	0	272	3147
5:10 PM	13	31	13	0	9	58	13	0	9	14	27	0	26	25	3	0	241	3156
5:15 PM	12	41	10	0	8	50	10	0	14	41	20	0	12	21	5	0	244	3135
5:20 PM	30	51	10	0	2	75	10	0	10	27	22	0	13	18	5	0	273	3172
5:25 PM	20	36	9	0	22	60	10	0	17	14	21	0	19	10	6	0	244	3159
5:30 PM	13	24	4	0	13	45	9	0	15	39	21	0	16	31	6	0	236	3132
5:35 PM	33	44	7	0	6	56	15	0	7	21	19	0	21	18	4	0	251	3108
5:40 PM	12	30	7	0	17	73	16	0	10	21	19	0	14	16	2	0	237	3070
5:45 PM	20	26	4	0	9	46	9	0	14	28	17	0	25	25	4	0	227	3038
5:50 PM	22	32	14	0	11	52	12	0	11	14	18	0	13	8	6	0	213	2983
5:55 PM	19	39	7	0	10	63	9	0	9	17	20	0	15	16	4	0	228	2944

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	272	464	140	4	152	800	104	0	172	352	280	0	204	260	64	0	3268
Heavy Trucks	24	36	4		8	16	0		0	24	16		8	4	8		148
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	1		0	0	0		0	0	0		1
Railroad																	
Stopped Buses																	

Comments:

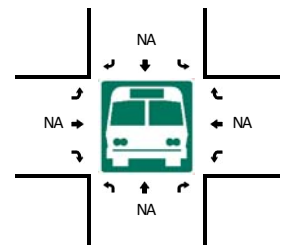
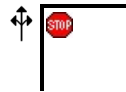
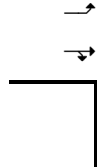
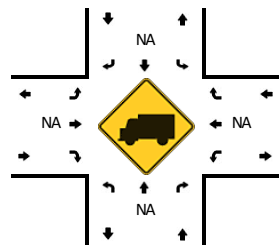
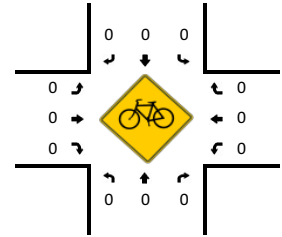
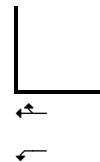
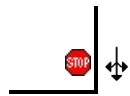
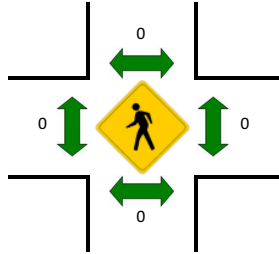
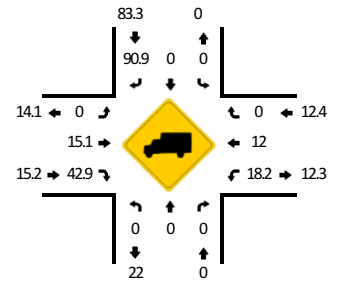
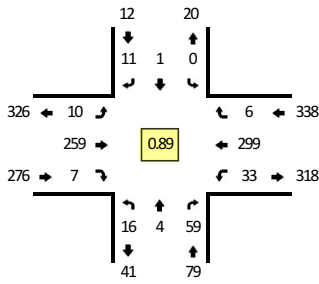
Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

LOCATION: Cooley Rd -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064703
DATE: Thu, Sep 12 2019

Peak-Hour: 6:40 AM -- 7:40 AM
Peak 15-Min: 7:10 AM -- 7:25 AM



5-Min Count Period Beginning At	Cooley Rd (Northbound)				Cooley Rd (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
6:00 AM	1	0	6	0	0	0	0	0	2	16	0	0	1	9	0	0	35	
6:05 AM	0	0	9	0	0	0	2	0	0	14	0	0	0	13	0	0	38	
6:10 AM	3	0	12	0	0	0	1	0	0	20	0	0	4	12	0	0	52	
6:15 AM	1	0	6	0	0	0	1	0	0	24	1	0	0	18	0	0	51	
6:20 AM	0	0	7	0	0	0	1	0	0	29	0	0	3	17	0	0	57	
6:25 AM	1	0	5	0	0	0	0	0	1	25	0	0	4	16	1	0	53	
6:30 AM	2	0	8	0	0	0	0	0	0	29	0	0	0	15	1	0	55	
6:35 AM	0	0	9	0	0	0	1	0	1	19	0	0	1	23	1	0	55	
6:40 AM	0	0	10	0	0	0	0	0	2	35	0	0	3	24	0	0	74	
6:45 AM	2	1	1	0	0	0	1	0	3	24	0	0	2	26	0	0	60	
6:50 AM	0	2	5	0	0	0	0	0	1	15	0	0	1	20	3	0	47	
6:55 AM	0	0	3	0	0	0	0	0	2	23	1	0	1	22	2	0	54	631
7:00 AM	2	0	6	0	0	0	0	0	1	16	1	0	2	17	1	0	46	642
7:05 AM	0	0	6	0	0	0	0	0	1	22	1	0	3	21	0	0	54	658
7:10 AM	0	1	5	0	0	0	1	0	0	23	1	0	2	30	0	0	63	669
7:15 AM	3	0	2	0	0	0	2	0	0	22	0	0	4	32	0	0	65	683
7:20 AM	1	0	5	0	0	0	4	0	0	22	0	0	6	32	0	0	70	696
7:25 AM	2	0	4	0	0	1	3	0	0	21	2	0	2	25	0	0	60	703
7:30 AM	0	0	9	0	0	0	0	0	0	16	1	0	5	22	0	0	53	701
7:35 AM	6	0	3	0	0	0	0	0	0	20	0	0	2	28	0	0	59	705
7:40 AM	0	0	3	0	0	0	0	0	1	15	0	0	1	29	0	0	49	680
7:45 AM	1	1	2	0	0	0	0	0	0	15	1	0	4	28	0	0	52	672
7:50 AM	2	0	4	0	0	0	1	0	0	17	2	0	1	25	0	0	52	677
7:55 AM	0	0	1	0	0	0	2	0	0	16	2	0	6	18	1	0	46	669
8:00 AM	1	0	1	0	0	0	0	0	1	15	1	0	0	17	0	0	36	659
8:05 AM	0	0	1	0	0	0	0	0	0	14	1	0	0	17	0	0	33	638
8:10 AM	0	0	2	0	0	0	0	0	0	12	2	0	4	14	0	0	34	609
8:15 AM	1	0	2	0	0	0	0	0	0	13	1	0	0	14	0	0	31	575
8:20 AM	1	0	0	0	0	0	0	0	0	16	3	0	5	14	0	0	39	544
8:25 AM	2	0	2	0	0	0	0	0	0	19	1	0	2	20	0	0	46	530
8:30 AM	2	0	2	0	0	0	0	0	0	13	1	0	2	19	0	0	39	516
8:35 AM	2	0	3	0	0	0	0	0	0	17	0	0	1	13	0	0	36	493
8:40 AM	3	0	0	0	0	0	1	0	0	17	0	0	3	9	0	0	33	477
8:45 AM	1	0	2	0	0	0	0	0	0	17	0	0	2	26	0	0	48	473
8:50 AM	0	0	2	0	0	0	2	0	0	11	1	0	2	12	0	0	30	451
8:55 AM	1	0	0	0	0	0	0	0	0	15	3	0	1	19	0	0	39	444

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	16	4	48	0	0	0	28	0	0	268	4	0	48	376	0	0	792
Heavy Trucks	0	0	0		0	0	28		0	36	0		4	44	0		112
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Railroad																	
Stopped Buses																	

Comments:

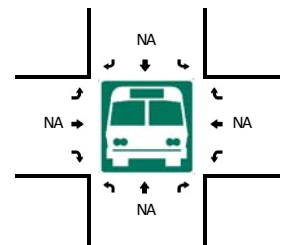
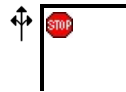
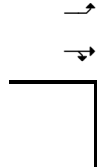
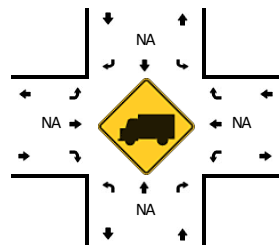
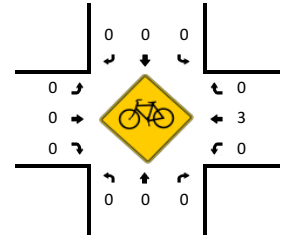
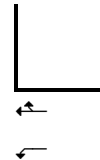
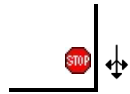
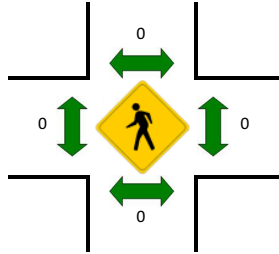
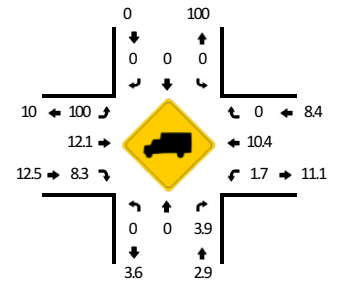
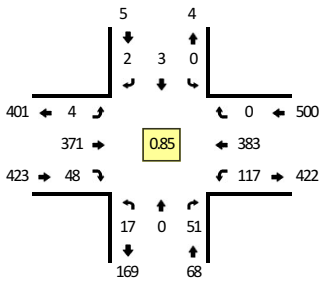
Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

LOCATION: Cooley Rd -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064704
DATE: Thu, Sep 12 2019

Peak-Hour: 3:45 PM -- 4:45 PM
Peak 15-Min: 4:15 PM -- 4:30 PM



5-Min Count Period Beginning At	Cooley Rd (Northbound)				Cooley Rd (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
3:00 PM	2	0	6	0	0	0	0	0	0	31	5	0	4	16	0	0	64	
3:05 PM	2	0	3	0	0	0	1	0	0	25	2	0	4	20	0	0	57	
3:10 PM	0	0	3	0	0	0	0	0	1	23	0	1	9	27	0	0	64	
3:15 PM	0	0	2	0	0	0	0	0	0	33	6	0	3	33	0	0	77	
3:20 PM	1	0	3	0	0	0	0	0	0	23	0	0	4	25	0	0	56	
3:25 PM	0	0	4	0	0	0	0	0	0	21	0	0	2	17	0	0	44	
3:30 PM	0	0	6	0	0	0	0	0	0	27	2	0	4	27	0	0	66	
3:35 PM	2	0	6	0	0	0	1	0	0	28	0	0	5	21	0	0	63	
3:40 PM	0	0	2	0	0	0	0	0	1	40	0	0	8	28	0	0	79	
3:45 PM	0	0	4	0	0	0	0	0	0	32	5	0	6	30	0	0	77	
3:50 PM	1	0	5	0	0	0	0	0	0	23	4	0	8	34	0	0	75	
3:55 PM	1	0	4	0	0	1	0	0	1	29	4	0	9	33	0	0	82	804
4:00 PM	1	0	5	0	0	0	0	0	1	35	2	0	8	23	0	0	75	815
4:05 PM	0	0	7	0	0	0	0	0	0	32	2	0	8	27	0	0	76	834
4:10 PM	3	0	7	0	0	0	0	0	0	28	1	0	9	32	0	0	80	850
4:15 PM	1	0	6	1	0	2	2	0	0	36	3	0	23	48	0	0	122	895
4:20 PM	1	0	1	0	0	0	0	0	0	36	7	0	8	37	0	0	90	929
4:25 PM	2	0	3	0	0	0	0	0	0	32	4	0	10	31	0	0	82	967
4:30 PM	2	0	3	0	0	0	0	0	1	32	5	0	6	23	0	0	72	973
4:35 PM	2	0	5	0	0	0	0	0	0	28	3	0	11	36	0	0	85	995
4:40 PM	2	0	1	0	0	0	0	0	1	28	8	0	11	29	0	0	80	996
4:45 PM	0	0	3	0	0	0	0	0	0	37	3	0	9	23	0	0	75	994
4:50 PM	1	0	8	0	0	0	0	0	1	25	4	0	10	14	0	1	64	983
4:55 PM	0	0	4	0	0	0	0	0	1	32	5	0	5	23	0	0	70	971
5:00 PM	1	0	4	0	0	0	0	0	0	35	8	0	6	29	0	0	83	979
5:05 PM	2	0	8	0	1	0	1	0	0	39	7	0	6	27	0	0	91	994
5:10 PM	0	0	11	0	0	1	1	0	0	24	1	0	7	29	0	0	74	988
5:15 PM	0	0	3	0	0	0	1	0	2	36	1	0	9	26	0	0	78	944
5:20 PM	1	0	5	0	0	0	0	0	0	33	4	0	6	17	0	0	66	920
5:25 PM	1	0	2	0	0	0	0	0	0	21	6	0	6	25	0	0	61	899
5:30 PM	2	0	9	0	0	0	0	0	1	32	5	0	2	28	0	0	79	906
5:35 PM	1	0	6	0	0	2	2	0	0	30	1	0	7	19	0	0	68	889
5:40 PM	2	0	4	0	1	0	1	0	0	23	2	0	3	21	0	0	57	866
5:45 PM	0	0	2	0	0	0	0	0	1	33	0	0	5	31	0	0	72	863
5:50 PM	2	0	4	0	0	0	0	0	0	27	4	0	2	8	0	0	47	846
5:55 PM	4	0	4	0	0	2	1	0	0	13	7	0	6	40	0	0	77	853

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	16	0	40	4	0	8	8	0	0	416	56	0	164	464	0	0	1176
Heavy Trucks	0	0	0		0	0	0		0	68	4		0	60	0		132
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Railroad																	
Stopped Buses																	

Comments:

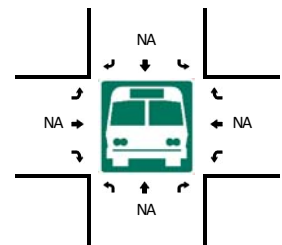
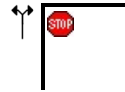
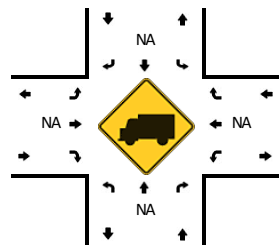
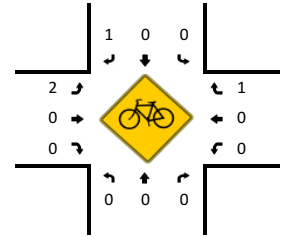
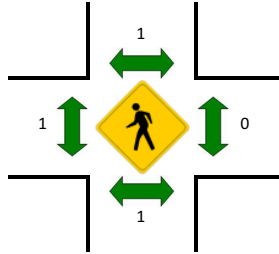
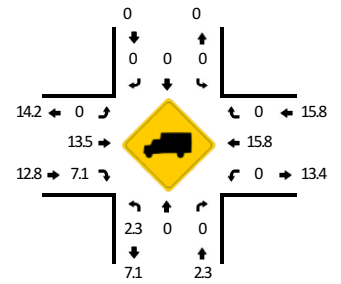
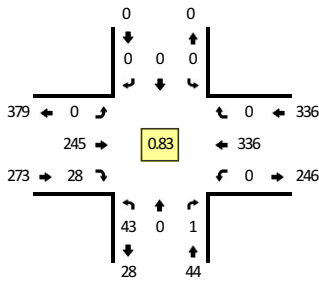
Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

LOCATION: June Way -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064705
DATE: Thu, Sep 12 2019

Peak-Hour: 7:05 AM -- 8:05 AM
Peak 15-Min: 7:10 AM -- 7:25 AM



5-Min Count Period Beginning At	June Way (Northbound)				June Way (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
6:00 AM	2	0	0	0	0	0	0	0	0	17	1	0	0	11	0	0	31	
6:05 AM	3	0	0	0	0	0	0	0	0	17	2	0	0	15	0	0	37	
6:10 AM	0	0	0	0	0	0	0	0	0	17	1	0	0	16	0	0	34	
6:15 AM	4	0	1	0	0	0	0	0	0	24	0	0	0	19	0	0	48	
6:20 AM	3	0	0	0	0	0	0	0	0	29	1	0	0	19	0	0	52	
6:25 AM	1	0	0	0	0	0	0	0	0	32	0	0	0	16	0	0	49	
6:30 AM	6	0	1	0	0	0	0	0	0	22	2	0	0	19	0	0	50	
6:35 AM	1	0	1	0	0	0	0	0	0	21	1	0	0	23	0	0	47	
6:40 AM	3	0	1	0	0	0	0	0	0	35	0	0	0	25	0	0	64	
6:45 AM	1	0	1	0	0	0	0	0	0	27	0	0	0	24	0	0	53	
6:50 AM	0	0	0	0	0	0	0	0	0	14	1	0	0	23	0	0	38	
6:55 AM	3	0	0	0	0	0	0	0	0	30	2	0	0	24	0	0	59	
7:00 AM	2	0	0	0	0	0	0	0	0	14	2	0	0	17	0	0	35	
7:05 AM	0	0	0	0	0	0	0	0	0	25	2	0	0	17	0	0	44	
7:10 AM	5	0	0	0	0	0	0	0	0	24	1	0	0	35	0	0	65	
7:15 AM	5	0	0	0	0	0	0	0	0	23	0	0	0	37	0	0	65	
7:20 AM	4	0	0	0	0	0	0	0	0	22	1	0	0	39	0	0	66	
7:25 AM	4	0	0	0	0	0	0	0	0	21	1	0	0	26	0	0	52	
7:30 AM	4	0	1	0	0	0	0	0	0	15	2	0	0	22	0	0	44	
7:35 AM	2	0	0	0	0	0	0	0	0	21	2	0	0	37	0	0	62	
7:40 AM	3	0	0	0	0	0	0	0	0	15	0	0	0	27	0	0	45	
7:45 AM	2	0	0	0	0	0	0	0	0	16	5	0	0	30	0	0	53	
7:50 AM	6	0	0	0	0	0	0	0	0	23	7	0	0	25	0	0	61	
7:55 AM	3	0	0	0	0	0	0	0	0	21	4	0	0	22	0	0	50	
8:00 AM	5	0	0	0	0	0	0	0	0	19	3	0	0	19	0	0	46	
8:05 AM	3	0	0	0	0	0	0	0	0	12	2	0	0	16	0	0	33	
8:10 AM	1	0	0	0	0	0	0	0	0	14	1	0	0	14	0	0	30	
8:15 AM	0	0	0	0	0	0	0	0	0	14	2	0	0	16	0	0	32	
8:20 AM	1	0	1	0	0	0	0	0	0	20	2	0	0	15	0	0	39	
8:25 AM	0	0	0	0	0	0	0	0	0	20	1	0	0	20	0	0	41	
8:30 AM	2	0	0	0	0	0	0	0	0	12	0	0	0	23	0	0	37	
8:35 AM	1	0	0	0	0	0	0	0	0	21	1	0	0	15	0	0	38	
8:40 AM	0	0	0	0	0	0	0	0	0	15	1	0	0	11	0	0	27	
8:45 AM	3	0	0	0	0	0	0	0	0	16	2	0	0	28	0	0	49	
8:50 AM	0	0	1	0	0	0	0	0	0	12	2	0	0	15	0	0	30	
8:55 AM	4	0	0	0	0	0	0	0	0	18	9	0	0	20	0	0	51	

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	56	0	0	0	0	0	0	0	0	276	8	0	0	444	0	0	784
Heavy Trucks	0	0	0		0	0	0		0	36	0		0	72	0		108
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Railroad																	
Stopped Buses																	

Comments:

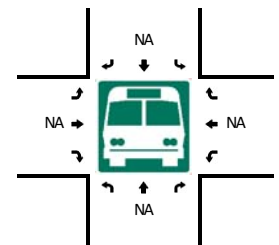
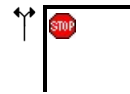
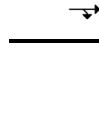
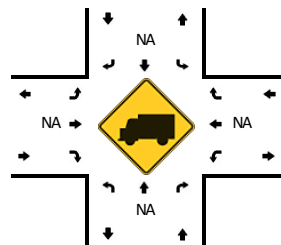
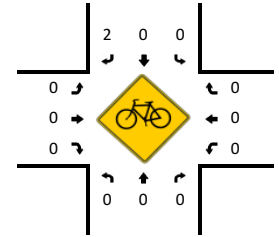
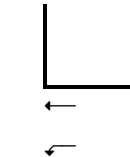
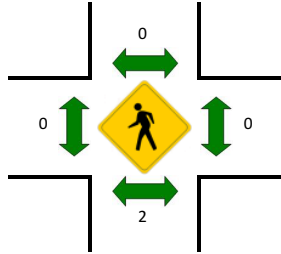
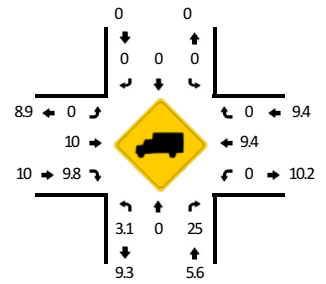
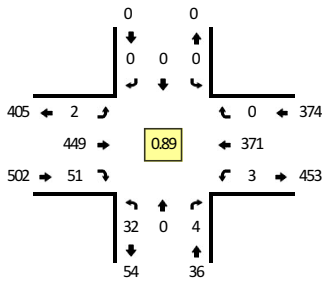
Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

LOCATION: June Way -- Molalla Rd
CITY/STATE: Marion, OR

QC JOB #: 15064706
DATE: Thu, Sep 12 2019

Peak-Hour: 4:10 PM -- 5:10 PM
Peak 15-Min: 4:15 PM -- 4:30 PM



5-Min Count Period Beginning At	June Way (Northbound)				June Way (Southbound)				Molalla Rd (Eastbound)				Molalla Rd (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
3:00 PM	2	0	0	0	0	0	0	0	0	0	35	5	0	0	16	0	0	58	
3:05 PM	3	0	1	0	0	0	0	0	0	0	28	2	0	0	21	0	0	56	
3:10 PM	6	0	0	0	0	0	0	0	0	0	28	2	0	0	27	0	0	63	
3:15 PM	2	0	1	0	0	0	0	0	0	0	34	5	0	0	32	0	0	74	
3:20 PM	3	0	0	0	0	0	0	0	0	0	23	6	0	1	29	0	0	62	
3:25 PM	3	0	0	0	0	0	0	0	0	0	20	5	0	0	15	0	0	43	
3:30 PM	1	0	1	0	0	0	0	0	0	0	30	4	0	0	27	0	0	63	
3:35 PM	0	0	0	0	0	0	0	0	0	0	27	3	0	0	25	0	0	55	
3:40 PM	1	0	0	0	0	0	0	0	0	0	40	4	0	2	20	0	0	67	
3:45 PM	1	0	0	0	0	0	0	0	0	0	38	1	0	0	37	0	0	77	
3:50 PM	1	0	0	0	0	0	0	0	0	0	27	6	0	0	36	0	0	70	
3:55 PM	2	0	0	0	0	0	0	0	0	0	39	4	0	0	31	0	0	76	764
4:00 PM	6	0	0	0	0	0	0	0	0	0	32	4	0	0	26	0	0	68	774
4:05 PM	1	0	0	0	0	0	0	0	0	0	36	5	0	0	28	0	0	70	788
4:10 PM	1	0	1	0	0	0	0	0	0	0	31	7	1	1	33	0	0	75	800
4:15 PM	2	0	0	0	0	0	0	0	0	0	37	3	0	1	46	0	0	89	815
4:20 PM	1	0	0	0	0	0	0	0	0	0	44	2	1	0	37	0	0	85	838
4:25 PM	4	0	0	0	0	0	0	0	0	0	36	5	0	1	37	0	0	83	878
4:30 PM	1	0	1	0	0	0	0	0	0	0	39	1	0	0	22	0	0	64	879
4:35 PM	2	0	1	0	0	0	0	0	0	0	29	6	0	0	38	0	0	76	900
4:40 PM	1	0	0	0	0	0	0	0	0	0	37	4	0	0	32	0	0	74	907
4:45 PM	4	0	0	0	0	0	0	0	0	0	41	6	0	0	24	0	0	75	905
4:50 PM	6	0	0	0	0	0	0	0	0	0	29	5	0	0	14	0	0	54	889
4:55 PM	2	0	0	0	0	0	0	0	0	0	38	4	0	0	23	0	0	67	880
5:00 PM	7	0	1	0	0	0	0	0	0	0	44	5	0	0	37	0	0	94	906
5:05 PM	1	0	0	0	0	0	0	0	0	0	44	3	0	0	28	0	0	76	912
5:10 PM	0	0	0	0	0	0	0	0	0	0	26	5	0	0	29	0	0	60	897
5:15 PM	1	0	1	0	0	0	0	0	0	0	40	3	0	0	27	0	0	72	880
5:20 PM	2	0	0	0	0	0	0	0	0	0	35	4	0	0	20	0	0	61	856
5:25 PM	3	0	0	0	0	0	0	0	0	0	30	4	0	0	24	0	0	61	834
5:30 PM	2	0	0	0	0	0	0	0	0	0	36	6	0	0	34	0	0	78	848
5:35 PM	5	0	0	0	0	0	0	0	0	0	30	3	0	0	21	0	0	59	831
5:40 PM	3	0	0	0	0	0	0	0	0	0	25	2	0	0	23	0	0	53	810
5:45 PM	1	0	0	0	0	0	0	0	0	0	33	2	0	0	33	0	0	69	804
5:50 PM	3	0	0	0	0	0	0	0	0	0	33	5	0	0	10	0	0	51	801
5:55 PM	6	0	0	0	0	0	0	0	0	0	20	6	0	0	40	0	0	72	806

Peak 15-Min Flowrates	Northbound				Southbound				Eastbound				Westbound				Total
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	
All Vehicles	28	0	0	0	0	0	0	0	0	468	40	4	8	480	0	0	1028
Heavy Trucks	0	0	0	0	0	0	0	0	0	64	4	0	0	60	0	0	128
Pedestrians		0				0				0				0			0
Bicycles	0	0	0		0	0	0		0	0	0		0	0	0		0
Railroad																	
Stopped Buses																	

Comments:

Report generated on 9/18/2019 2:50 PM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212


























APPENDIX C:
SYNCHRO REPORTS – 2019 EXISTING AM + PM

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	120	190	115	150	235	95	170	510	100	70	340	80
Future Volume (vph)	120	190	115	150	235	95	170	510	100	70	340	80
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.96		1.00	1.00	0.85	1.00	0.97	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1471	1549	1298	1471	1476		2906	2995	1311	1484	2877	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1471	1549	1298	1471	1476		2906	2995	1311	1484	2877	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	133	211	128	167	261	106	189	567	111	78	378	89
RTOR Reduction (vph)	0	0	100	0	14	0	0	0	76	0	20	0
Lane Group Flow (vph)	133	211	28	167	353	0	189	567	35	78	447	0
Confl. Peds. (#/hr)	1		3	3		1	1		1	1		1
Heavy Vehicles (%)	13%	13%	13%	13%	13%	13%	11%	11%	11%	12%	12%	12%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	11.7	19.8	19.8	13.9	22.0		10.8	28.0	28.0	8.3	25.5	
Effective Green, g (s)	11.7	19.8	19.8	13.9	22.0		10.8	28.0	28.0	8.3	25.5	
Actuated g/C Ratio	0.13	0.22	0.22	0.15	0.24		0.12	0.31	0.31	0.09	0.28	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	191	340	285	227	360		348	931	407	136	815	
v/s Ratio Prot	0.09	0.14		c0.11	c0.24		c0.07	c0.19		0.05	0.16	
v/s Ratio Perm			0.02						0.03			
v/c Ratio	0.70	0.62	0.10	0.74	0.98		0.54	0.61	0.08	0.57	0.55	
Uniform Delay, d1	37.5	31.7	28.0	36.3	33.8		37.3	26.3	21.9	39.2	27.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	10.5	3.6	0.2	11.7	42.4		1.7	1.7	0.2	5.7	1.4	
Delay (s)	48.0	35.3	28.1	48.0	76.2		39.0	28.1	22.1	44.9	28.8	
Level of Service	D	D	C	D	E		D	C	C	D	C	
Approach Delay (s)		36.9			67.4			29.7			31.1	
Approach LOS		D			E			C			C	
Intersection Summary												
HCM 2000 Control Delay			39.7			HCM 2000 Level of Service			D			
HCM 2000 Volume to Capacity ratio			0.77									
Actuated Cycle Length (s)			90.0			Sum of lost time (s)			20.0			
Intersection Capacity Utilization			63.7%			ICU Level of Service			B			
Analysis Period (min)			15									

c Critical Lane Group

Queuing and Blocking Report
Baseline

11/23/2019

Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	NB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	R	L	T
Maximum Queue (ft)	353	864	13	324	2026	306	359	1572	1571	16	389	1811
Average Queue (ft)	337	838	0	276	2009	296	355	1421	1184	1	385	1348
95th Queue (ft)	372	864	7	373	2111	378	374	1938	2169	13	418	2189
Link Distance (ft)		834			2018			1552	1552			1782
Upstream Blk Time (%)		100			92			77	60			37
Queuing Penalty (veh)		0			369			0	0			0
Storage Bay Dist (ft)	350		550	300		350	350			230	380	
Storage Blk Time (%)	77	30		76	51	40	99				95	0
Queuing Penalty (veh)	235	74		244	78	101	252				161	0
























Intersection: 1: OR 99E & Molalla Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	1796
Average Queue (ft)	1326
95th Queue (ft)	2173
Link Distance (ft)	1782
Upstream Blk Time (%)	35
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	160	350	265	215	285	75	255	495	135	160	815	125
Future Volume (vph)	160	350	265	215	285	75	255	495	135	160	815	125
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1554	1636	1365	1554	1580		3014	3107	1359	1568	3068	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1554	1636	1365	1554	1580		3014	3107	1359	1568	3068	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	167	365	276	224	297	78	266	516	141	167	849	130
RTOR Reduction (vph)	0	0	210	0	7	0	0	0	98	0	9	0
Lane Group Flow (vph)	167	365	66	224	368	0	266	516	43	167	970	0
Confl. Peds. (#/hr)	1		6	6		1	2		1	1		2
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	6%	6%	6%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	18.1	29.7	29.7	21.8	33.4		14.2	37.8	37.8	15.1	38.7	
Effective Green, g (s)	18.1	29.7	29.7	21.8	33.4		14.2	37.8	37.8	15.1	38.7	
Actuated g/C Ratio	0.15	0.24	0.24	0.18	0.27		0.11	0.30	0.30	0.12	0.31	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	226	390	325	272	424		344	944	412	190	954	
v/s Ratio Prot	0.11	0.22		c0.14	c0.23		0.09	0.17		c0.11	c0.32	
v/s Ratio Perm			0.05						0.03			
v/c Ratio	0.74	0.94	0.20	0.82	0.87		0.77	0.55	0.10	0.88	1.02	
Uniform Delay, d1	50.9	46.4	37.9	49.4	43.4		53.5	36.1	31.1	53.8	42.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	11.9	29.7	0.3	17.9	17.1		10.3	1.2	0.2	33.7	33.4	
Delay (s)	62.8	76.1	38.2	67.4	60.5		63.9	37.4	31.4	87.4	76.3	
Level of Service	E	E	D	E	E		E	D	C	F	E	
Approach Delay (s)		60.4			63.1			44.1			77.9	
Approach LOS		E			E			D			E	
Intersection Summary												
HCM 2000 Control Delay			62.3				HCM 2000 Level of Service			E		
HCM 2000 Volume to Capacity ratio			0.94									
Actuated Cycle Length (s)			124.4				Sum of lost time (s)			20.0		
Intersection Capacity Utilization			88.3%				ICU Level of Service			E		
Analysis Period (min)			15									
c Critical Lane Group												

Queuing and Blocking Report
Baseline

11/23/2019

Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	NB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	R	L	T
Maximum Queue (ft)	371	884	458	325	1626	275	362	1581	1536	9	370	1799
Average Queue (ft)	333	844	30	158	862	275	362	1529	975	1	371	1709
95th Queue (ft)	445	877	234	381	2089	344	370	1709	2179	7	406	2036
Link Distance (ft)		834			2018			1552	1552			1782
Upstream Blk Time (%)		99			12			91	53			84
Queuing Penalty (veh)		0			53			0	0			0
Storage Bay Dist (ft)	350		550	300		350	350			230	380	
Storage Blk Time (%)	74	28	0	1	38	20	100				60	
Queuing Penalty (veh)	453	122	0	2	82	50	248				245	

Intersection: 1: OR 99E & Molalla Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	1799
Average Queue (ft)	1700
95th Queue (ft)	2047
Link Distance (ft)	1782
Upstream Blk Time (%)	83
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection						
Int Delay, s/veh	1.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	260	30	1	355	50	5
Future Vol, veh/h	260	30	1	355	50	5
Conflicting Peds, #/hr	0	1	1	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	13	13	16	16	2	2
Mvmt Flow	313	36	1	428	60	6

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	350	0	763 332
Stage 1	-	-	-	-	332 -
Stage 2	-	-	-	-	431 -
Critical Hdwy	-	-	4.26	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.344	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1135	-	372 710
Stage 1	-	-	-	-	727 -
Stage 2	-	-	-	-	655 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1134	-	371 709
Mov Cap-2 Maneuver	-	-	-	-	371 -
Stage 1	-	-	-	-	726 -
Stage 2	-	-	-	-	654 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	16.2
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	388	-	-	1134	-
HCM Lane V/C Ratio	0.171	-	-	0.001	-
HCM Control Delay (s)	16.2	-	-	8.2	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.6	-	-	0	-

HCM 6th TWSC
2: June Way & Molalla Rd

11/23/2019

Intersection						
Int Delay, s/veh	0.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	475	55	5	390	35	5
Future Vol, veh/h	475	55	5	390	35	5
Conflicting Peds, #/hr	0	2	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	10	10	9	9	6	6
Mvmt Flow	534	62	6	438	39	6

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	598	0	1017 567
Stage 1	-	-	-	-	567 -
Stage 2	-	-	-	-	450 -
Critical Hdwy	-	-	4.19	-	6.46 6.26
Critical Hdwy Stg 1	-	-	-	-	5.46 -
Critical Hdwy Stg 2	-	-	-	-	5.46 -
Follow-up Hdwy	-	-	2.281	-	3.554 3.354
Pot Cap-1 Maneuver	-	-	945	-	259 515
Stage 1	-	-	-	-	560 -
Stage 2	-	-	-	-	634 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	943	-	257 514
Mov Cap-2 Maneuver	-	-	-	-	257 -
Stage 1	-	-	-	-	559 -
Stage 2	-	-	-	-	630 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	20.7
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	274	-	-	943	-
HCM Lane V/C Ratio	0.164	-	-	0.006	-
HCM Control Delay (s)	20.7	-	-	8.8	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.6	-	-	0	-

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	5	240	15	40	325	5	20	5	50	1	5	15
Future Vol, veh/h	5	240	15	40	325	5	20	5	50	1	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	14	14	14	12	12	12	0	0	0	93	93	93
Mvmt Flow	6	289	18	48	392	6	24	6	60	1	6	18

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	398	0	0	307	0	0	813	804	298	834	810	395
Stage 1	-	-	-	-	-	-	310	310	-	491	491	-
Stage 2	-	-	-	-	-	-	503	494	-	343	319	-
Critical Hdwy	4.24	-	-	4.22	-	-	7.1	6.5	6.2	8.03	7.43	7.13
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Follow-up Hdwy	2.326	-	-	2.308	-	-	3.5	4	3.3	4.337	4.837	4.137
Pot Cap-1 Maneuver	1098	-	-	1199	-	-	299	319	746	205	230	495
Stage 1	-	-	-	-	-	-	705	663	-	422	423	-
Stage 2	-	-	-	-	-	-	555	550	-	519	518	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1098	-	-	1199	-	-	272	305	746	179	220	495
Mov Cap-2 Maneuver	-	-	-	-	-	-	272	305	-	179	220	-
Stage 1	-	-	-	-	-	-	701	660	-	420	406	-
Stage 2	-	-	-	-	-	-	506	528	-	470	515	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.9			14.3			15.8		
HCM LOS							B			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	478	1098	-	-	1199	-	-	358
HCM Lane V/C Ratio	0.189	0.005	-	-	0.04	-	-	0.071
HCM Control Delay (s)	14.3	8.3	-	-	8.1	-	-	15.8
HCM Lane LOS	B	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.2

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	3.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	5	410	65	120	370	1	20	1	60	5	5	5
Future Vol, veh/h	5	410	65	120	370	1	20	1	60	5	5	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	11	11	11	7	7	7	0	0	0	0	0	0
Mvmt Flow	6	482	76	141	435	1	24	1	71	6	6	6

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	436	0	0	558	0	0	1256	1250	520	1286	1288	436
Stage 1	-	-	-	-	-	-	532	532	-	718	718	-
Stage 2	-	-	-	-	-	-	724	718	-	568	570	-
Critical Hdwy	4.21	-	-	4.17	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.299	-	-	2.263	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1077	-	-	988	-	-	150	174	560	143	165	625
Stage 1	-	-	-	-	-	-	535	529	-	423	436	-
Stage 2	-	-	-	-	-	-	420	436	-	511	509	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1077	-	-	988	-	-	128	148	560	110	141	625
Mov Cap-2 Maneuver	-	-	-	-	-	-	128	148	-	110	141	-
Stage 1	-	-	-	-	-	-	532	526	-	420	374	-
Stage 2	-	-	-	-	-	-	351	374	-	443	506	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			2.3			22.5			28.8		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	300	1077	-	-	988	-	-	169
HCM Lane V/C Ratio	0.318	0.005	-	-	0.143	-	-	0.104
HCM Control Delay (s)	22.5	8.4	-	-	9.2	-	-	28.8
HCM Lane LOS	C	A	-	-	A	-	-	D
HCM 95th %tile Q(veh)	1.3	0	-	-	0.5	-	-	0.3



**APPENDIX D:
METHODS AND ASSUMPTIONS MEMORANDUM**

MEMORANDUM

DATE: September 26, 2019

TO: Eric Lilequist/City of Woodburn, Dago Garcia/ City of Woodburn,
Casey Knecht/ODOT Region 2

FROM: Tegan Enloe, PE/Enloe Consulting, LLC

SUBJECT: Molalla Road Apartments Traffic Impact Analysis: Methods & Assumptions

Background

The applicant is proposing to build an apartment complex located on the north side of Molalla Road just east of N Pacific Highway (Hwy) at 2145 Molalla Road. The proposed development would build two-hundred thirty-one (231) apartment units. The complex would have direct access to Molalla Road.

Project Study Area

The project study area is outlined in Figure 1. Study intersections are identified with circles on the figure and include:

1. N Pacific Hwy (99E) / Molalla Road (OR 211)
2. Molalla Road (OR 211) / June Way
3. Molalla Road (OR 211) / Proposed Site Driveway
4. Molalla Road (OR 211) / Cooley Road

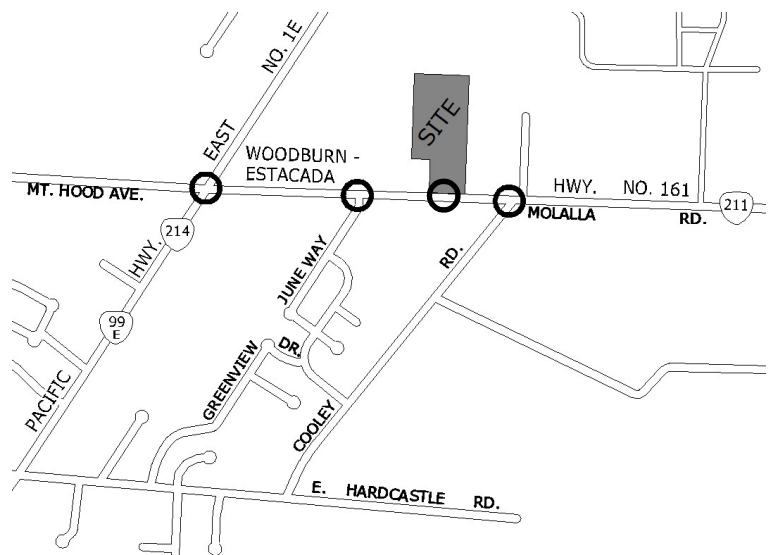


Figure 1: Study Area



Table 1 outlines study intersection ownership and applicable mobility standards.

Table 1: Intersection Mobility Standards and Ownership

No.	Intersection	Ownership ¹	Functional Classification ²	Mobility Standard Type	Standard	
					Major	Minor
1	N Pacific Hwy (99E) / Molalla Road (OR 211)	ODOT	Principal Arterial/ Minor Arterial	V/C	0.90	
2	Molalla Road (OR 211) / June Way	ODOT	Principal Arterial/ Local	V/C	0.95	1.0
3	Molalla Road (OR 211) / Proposed Site Driveway	ODOT	Minor Arterial/ Local	V/C	0.95	1.0
4	Molalla Road (OR 211) / Cooley Road	ODOT	Principal Arterial/ Major Collector	V/C	0.95	0.95

Notes:
 ODOT Standards based on OHP, Table 6 for Regional Highway, non-MPO less than or equal to 35 mph
 Molalla Road (OR 211) is also known as Woodburn-Estacada Hwy No. 161
 N Pacific Hwy (O 99E) is also known as Pacific Hwy No. 081

¹ Ownership, ODOT City of Woodburn Map, Accessed 08/09/2019
https://www.oregon.gov/ODOT/Data/Documents/City_Woodburn.pdf

² Functional Classification, ODOT TransGIS, Accessed 08/09/2019

Scenarios

The traffic analysis will be completed under the assumption that full build out and site occupancy will occur in 2021.

The following analysis scenarios are proposed for review:

- 2019 Existing Conditions, AM Peak Hour
- 2019 Existing Conditions, PM Peak Hour
- 2021 Background Traffic, AM Peak Hour
- 2021 Background Traffic, PM Peak Hour
- 2021 Total Traffic (Background + Site), AM Peak Hour
- 2021 Total Traffic (Background + Site), PM Peak Hour

A simulation-based queue length analysis will be provided for any intersections that exceed 0.70 volume-to-capacity ratio.

Analysis Volumes

AM and PM traffic counts for the study area were conducted on Thursday, September 12th, 2019. The intersection of North Pacific Highway (OR 99E)/Molalla Rd is separated from the remaining study area by a large driveway. The remaining intersections are separated by a few minor driveways. As such, the network will only be balanced between the intersections Molalla Road/June Way and Molalla Road (OR 211) /Cooley Road. 30th Highest Volume (HV) values will be used for all analysis scenarios.

Seasonal Adjustment Factor

To establish the 30th HV values for the analysis, the seasonal adjustment factor is calculated using the ODOT Characteristic Table, which identifies ATR 24-001 (Woodburn) as a representative ATR for the study area. Table 2 shows the seasonal adjustment factor calculations.



Table 2: Seasonal Adjustment Factor Calculation

	2017	2016	2015	2014	2013	Average
Peak Month	117 (August)	111 (June)	113 (June)	112 (July)	112 (July)	112
Count Month (September)	109	106	105	109	107	107
Average Seasonal Adjustment Factor: 1.05						
Note: strike through values represent the highest and lowest values and were excluded from the average.						

Future Forecasting

A growth rate for the study area is developed using ODOT’s 2037 Future Volume Tables. Table 3 outlines the proposed Future Volume Table data points. An annual growth rate of 1.4% is proposed to be applied linearly to the existing volumes. This value is achieved by average of the two calculated values for North Pacific Highway (OR 99E), which represents the highest order roadway in the study area.

Table 3: Future Volume Table

Highway	Mile Point	Location	2016	2037	RSQ	21 Year Factor	Annual Rate
081 (OR99E)	31.65	0.05 mile north of Woodburn-Estacada Highway (OR211) and Hillsboro-Silverton Highway (OR214)	18300	22300	MODEL	1.22	1.0%
081 (OR99E)	31.80	0.10 mile south of Woodburn-Estacada Highway (OR211)	19500	26600	MODEL	1.36	1.7%
140	39.24	0.05 mile west of Pacific Highway East (OR99E)	13800	15800	MODEL	1.14	0.7%
161	0.15	0.15 mile east of Pacific Highway East (OR99E) and Hillsboro-Silverton Highway	8800	12300	0.5248	1.40	1.9%
Average Annual Rate: 1.4%							



Trip Generation

Trip generation values for the proposed development are provided in Table 4. These values will be added to the 2021 Background Traffic to develop 2021 Total Traffic volumes.

Table 4: Trip Generation

Land Use (ITE Codes)	Dwelling Units	Time Period	Trip Generation Rate	Peak Hour Trips		
				In	Out	Total
Multi-Family Mid-Rise (221)	231	AM Peak	Equation	20	58	78
TOTAL AM PEAK HOUR				20	58	78
Multi-Family Mid-Rise (221)	231	PM Peak	Equation	61	38	99
TOTAL PM PEAK HOUR				61	38	99

Trip Distribution

Trip distribution patterns are calculated based on a combination of existing count data and recommendations from the City and ODOT.

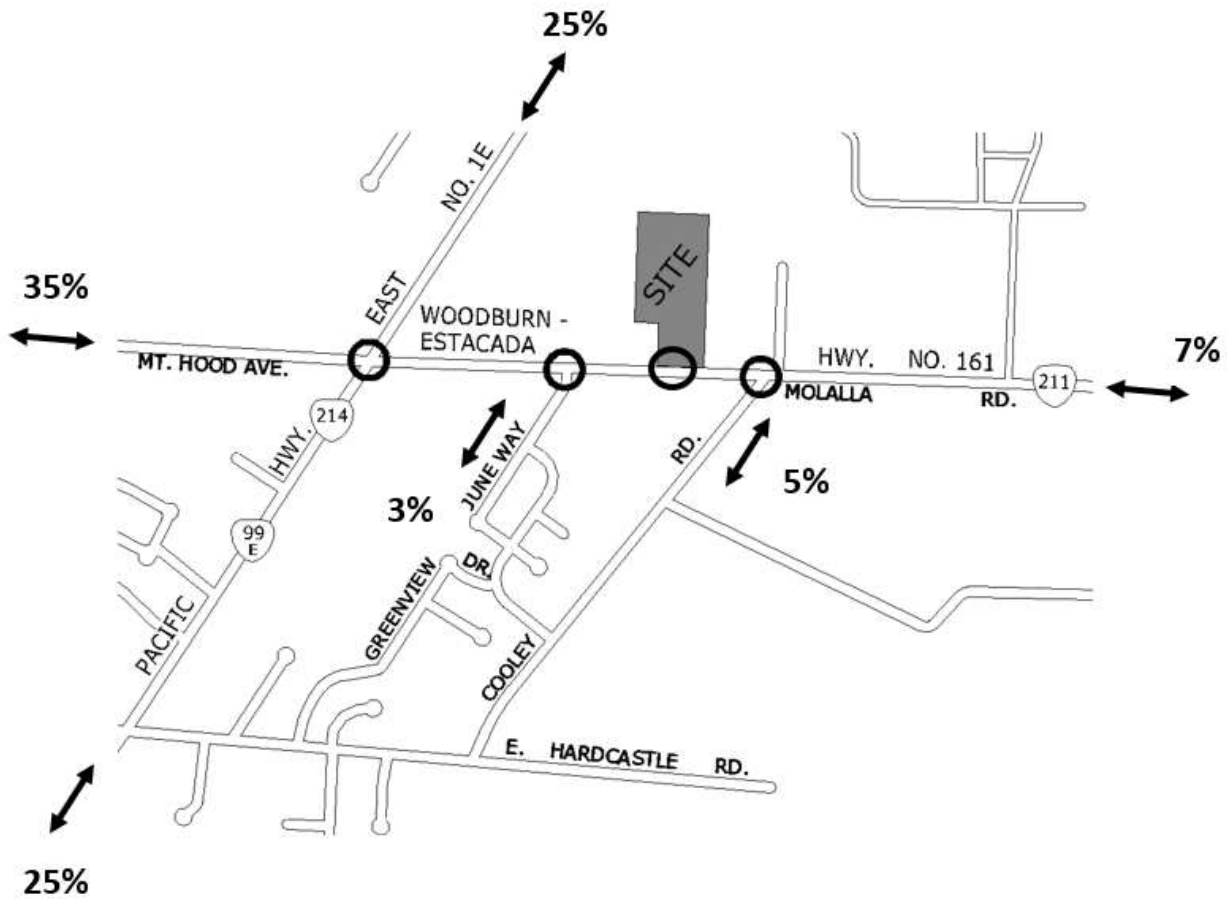


Figure 2: Proposed Trip Distribution

In Process Developments

Conversations with City and ODOT staff have indicated that trips at the intersection of North Pacific Highway (OR 99E)/Molalla Road from the Pacific Valley Apartments should be included as “in-process”. In process trips will be included in the Background and Total Traffic analysis scenarios.

Next Steps

If you have any questions or comments regarding the proposed approach, please contact our Project Manager, Tegan Enloe, PE, at 503.805.8115 or enloeconsulting@gmail.com.



APPENDIX E: IN-PROCESS TRIPS

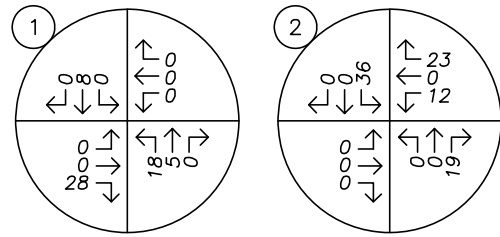
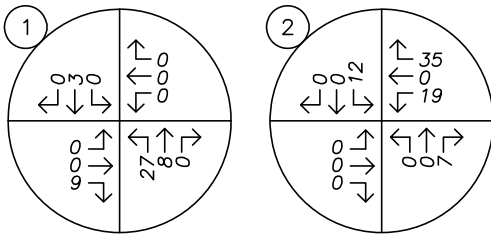
LEGEND

XX% PERCENT OF PROJECT TRIPS

TRIP GENERATION			
	IN	OUT	TOTAL
AM	19	54	73
PM	55	35	90

AM PEAK HOUR

PM PEAK HOUR



SITE TRIP DISTRIBUTION & ASSIGNMENT
 Proposed Development Plan – Site Trips
 AM & PM Peak Hours





APPENDIX F:
SYNCHRO REPORTS – 2021 BACKGROUND AM + PM

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	120	195	130	155	240	95	205	530	105	75	350	80
Future Volume (vph)	120	195	130	155	240	95	205	530	105	75	350	80
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.96		1.00	1.00	0.85	1.00	0.97	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1471	1549	1298	1471	1477		2906	2995	1311	1484	2879	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1471	1549	1298	1471	1477		2906	2995	1311	1484	2879	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	133	217	144	172	267	106	228	589	117	83	389	89
RTOR Reduction (vph)	0	0	113	0	13	0	0	0	80	0	19	0
Lane Group Flow (vph)	133	217	31	172	360	0	228	589	37	83	459	0
Confl. Peds. (#/hr)	1		3	3		1	1		1	1		1
Heavy Vehicles (%)	13%	13%	13%	13%	13%	13%	11%	11%	11%	12%	12%	12%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	11.8	19.5	19.5	14.2	21.9		12.1	29.2	29.2	8.6	25.7	
Effective Green, g (s)	11.8	19.5	19.5	14.2	21.9		12.1	29.2	29.2	8.6	25.7	
Actuated g/C Ratio	0.13	0.21	0.21	0.16	0.24		0.13	0.32	0.32	0.09	0.28	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	189	330	276	228	353		384	955	418	139	808	
v/s Ratio Prot	0.09	0.14		c0.12	c0.24		c0.08	c0.20		0.06	0.16	
v/s Ratio Perm			0.02						0.03			
v/c Ratio	0.70	0.66	0.11	0.75	1.02		0.59	0.62	0.09	0.60	0.57	
Uniform Delay, d1	38.2	32.9	29.0	37.0	34.8		37.4	26.4	21.8	39.8	28.1	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	11.3	4.8	0.2	13.2	53.1		2.5	1.8	0.2	6.7	1.6	
Delay (s)	49.4	37.7	29.2	50.2	87.9		39.8	28.2	22.0	46.5	29.7	
Level of Service	D	D	C	D	F		D	C	C	D	C	
Approach Delay (s)		38.4			76.0			30.3			32.2	
Approach LOS		D			E			C			C	
Intersection Summary												
HCM 2000 Control Delay			42.1				HCM 2000 Level of Service				D	
HCM 2000 Volume to Capacity ratio			0.80									
Actuated Cycle Length (s)			91.5			Sum of lost time (s)				20.0		
Intersection Capacity Utilization			64.8%			ICU Level of Service				C		
Analysis Period (min)			15									

c Critical Lane Group

Queuing and Blocking Report
Baseline

11/23/2019

Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	NB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	R	L	T
Maximum Queue (ft)	362	868	401	320	2030	280	352	1594	1580	1	380	1826
Average Queue (ft)	340	844	30	256	2013	280	352	1513	1119	0	379	1419
95th Queue (ft)	428	895	235	408	2092	345	371	1778	2068	0	426	2147
Link Distance (ft)		834			2018			1552	1552			1782
Upstream Blk Time (%)		97			94			87	34			40
Queuing Penalty (veh)		0			386			0	0			0
Storage Bay Dist (ft)	350		550	300		350	350			230	380	
Storage Blk Time (%)	88	14	0	66	41	20	100				80	4
Queuing Penalty (veh)	284	37	0	220	65	53	265				139	4

Intersection: 1: OR 99E & Molalla Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	1791
Average Queue (ft)	1384
95th Queue (ft)	2120
Link Distance (ft)	1782
Upstream Blk Time (%)	38
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	165	360	300	225	290	80	280	515	135	165	850	125
Future Volume (vph)	165	360	300	225	290	80	280	515	135	165	850	125
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1554	1636	1365	1554	1578		3014	3107	1359	1568	3070	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1554	1636	1365	1554	1578		3014	3107	1359	1568	3070	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	172	375	312	234	302	83	292	536	141	172	885	130
RTOR Reduction (vph)	0	0	224	0	7	0	0	0	98	0	9	0
Lane Group Flow (vph)	172	375	89	234	378	0	292	536	43	172	1006	0
Confl. Peds. (#/hr)	1		6	6		1	2		1	1		2
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	6%	6%	6%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	18.6	30.5	30.5	22.5	34.4		14.8	38.1	38.1	15.3	38.6	
Effective Green, g (s)	18.6	30.5	30.5	22.5	34.4		14.8	38.1	38.1	15.3	38.6	
Actuated g/C Ratio	0.15	0.24	0.24	0.18	0.27		0.12	0.30	0.30	0.12	0.31	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	228	394	329	276	429		352	936	409	189	937	
v/s Ratio Prot	0.11	0.23		c0.15	c0.24		0.10	0.17		c0.11	c0.33	
v/s Ratio Perm			0.07						0.03			
v/c Ratio	0.75	0.95	0.27	0.85	0.88		0.83	0.57	0.10	0.91	1.07	
Uniform Delay, d1	51.7	47.2	38.9	50.3	44.0		54.6	37.3	31.8	54.9	43.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	13.2	33.0	0.5	20.7	18.9		14.8	1.4	0.3	41.0	51.2	
Delay (s)	64.9	80.3	39.4	71.0	63.0		69.4	38.7	32.1	95.9	95.1	
Level of Service	E	F	D	E	E		E	D	C	F	F	
Approach Delay (s)		62.3			66.0			47.0			95.2	
Approach LOS		E			E			D			F	
Intersection Summary												
HCM 2000 Control Delay			69.6				HCM 2000 Level of Service			E		
HCM 2000 Volume to Capacity ratio			0.97									
Actuated Cycle Length (s)			126.4				Sum of lost time (s)			20.0		
Intersection Capacity Utilization			91.2%				ICU Level of Service			F		
Analysis Period (min)			15									
c Critical Lane Group												

Queuing and Blocking Report
Baseline

11/23/2019

Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	L	T	TR
Maximum Queue (ft)	364	860	352	325	1574	281	359	1576	1552	390	1810	1815
Average Queue (ft)	345	833	25	131	817	275	357	1543	1469	391	1682	1689
95th Queue (ft)	410	848	215	338	2108	336	384	1634	1702	397	2089	2124
Link Distance (ft)		834			2018			1552	1552		1782	1782
Upstream Blk Time (%)		99			16			74	55		81	79
Queuing Penalty (veh)		0			71			0	0		0	0
Storage Bay Dist (ft)	350		550	300		350	350			380		
Storage Blk Time (%)	87	16	0	1	34	0	79			100	0	
Queuing Penalty (veh)	572	78	0	2	77	0	203			425	0	

Intersection						
Int Delay, s/veh	1.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	265	30	1	365	50	5
Future Vol, veh/h	265	30	1	365	50	5
Conflicting Peds, #/hr	0	1	1	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	13	13	16	16	2	2
Mvmt Flow	319	36	1	440	60	6

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	356	0	781 338
Stage 1	-	-	-	-	338 -
Stage 2	-	-	-	-	443 -
Critical Hdwy	-	-	4.26	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.344	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1129	-	363 704
Stage 1	-	-	-	-	722 -
Stage 2	-	-	-	-	647 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1128	-	362 703
Mov Cap-2 Maneuver	-	-	-	-	362 -
Stage 1	-	-	-	-	721 -
Stage 2	-	-	-	-	646 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	16.5
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	379	-	-	1128	-
HCM Lane V/C Ratio	0.175	-	-	0.001	-
HCM Control Delay (s)	16.5	-	-	8.2	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.6	-	-	0	-

Intersection						
Int Delay, s/veh	0.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	485	55	5	405	35	5
Future Vol, veh/h	485	55	5	405	35	5
Conflicting Peds, #/hr	0	2	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	10	10	9	9	6	6
Mvmt Flow	545	62	6	455	39	6

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	609	0	1045
Stage 1	-	-	-	-	578
Stage 2	-	-	-	-	467
Critical Hdwy	-	-	4.19	-	6.46
Critical Hdwy Stg 1	-	-	-	-	5.46
Critical Hdwy Stg 2	-	-	-	-	5.46
Follow-up Hdwy	-	-	2.281	-	3.554
Pot Cap-1 Maneuver	-	-	936	-	249
Stage 1	-	-	-	-	553
Stage 2	-	-	-	-	623
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	934	-	247
Mov Cap-2 Maneuver	-	-	-	-	247
Stage 1	-	-	-	-	552
Stage 2	-	-	-	-	619

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	21.4
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	264	-	-	934	-
HCM Lane V/C Ratio	0.17	-	-	0.006	-
HCM Control Delay (s)	21.4	-	-	8.9	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.6	-	-	0	-

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↶	↷		↶	↷			↕			↕	
Traffic Vol, veh/h	5	245	15	40	335	5	20	5	50	1	5	15
Future Vol, veh/h	5	245	15	40	335	5	20	5	50	1	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	14	14	14	12	12	12	0	0	0	93	93	93
Mvmt Flow	6	295	18	48	404	6	24	6	60	1	6	18

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	410	0	0	313	0	0	831	822	304	852	828	407
Stage 1	-	-	-	-	-	-	316	316	-	503	503	-
Stage 2	-	-	-	-	-	-	515	506	-	349	325	-
Critical Hdwy	4.24	-	-	4.22	-	-	7.1	6.5	6.2	8.03	7.43	7.13
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Follow-up Hdwy	2.326	-	-	2.308	-	-	3.5	4	3.3	4.337	4.837	4.137
Pot Cap-1 Maneuver	1087	-	-	1193	-	-	291	311	740	198	223	487
Stage 1	-	-	-	-	-	-	699	659	-	414	417	-
Stage 2	-	-	-	-	-	-	546	543	-	514	514	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1087	-	-	1193	-	-	265	297	740	173	213	487
Mov Cap-2 Maneuver	-	-	-	-	-	-	265	297	-	173	213	-
Stage 1	-	-	-	-	-	-	695	655	-	412	400	-
Stage 2	-	-	-	-	-	-	497	521	-	465	511	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.9			14.5			16.1		
HCM LOS							B			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	469	1087	-	-	1193	-	-	350
HCM Lane V/C Ratio	0.193	0.006	-	-	0.04	-	-	0.072
HCM Control Delay (s)	14.5	8.3	-	-	8.1	-	-	16.1
HCM Lane LOS	B	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.2

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	3.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	5	420	65	125	380	1	20	1	60	5	5	5
Future Vol, veh/h	5	420	65	125	380	1	20	1	60	5	5	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	11	11	11	7	7	7	0	0	0	0	0	0
Mvmt Flow	6	494	76	147	447	1	24	1	71	6	6	6

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	448	0	0	570	0	0	1292	1286	532	1322	1324	448
Stage 1	-	-	-	-	-	-	544	544	-	742	742	-
Stage 2	-	-	-	-	-	-	748	742	-	580	582	-
Critical Hdwy	4.21	-	-	4.17	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.299	-	-	2.263	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1066	-	-	978	-	-	141	166	551	135	157	615
Stage 1	-	-	-	-	-	-	527	522	-	411	425	-
Stage 2	-	-	-	-	-	-	408	425	-	504	502	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1066	-	-	978	-	-	119	140	551	103	133	615
Mov Cap-2 Maneuver	-	-	-	-	-	-	119	140	-	103	133	-
Stage 1	-	-	-	-	-	-	524	519	-	409	361	-
Stage 2	-	-	-	-	-	-	338	361	-	436	499	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			2.3			23.8			30.4		
HCM LOS							C			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	285	1066	-	-	978	-	-	159
HCM Lane V/C Ratio	0.334	0.006	-	-	0.15	-	-	0.111
HCM Control Delay (s)	23.8	8.4	-	-	9.3	-	-	30.4
HCM Lane LOS	C	A	-	-	A	-	-	D
HCM 95th %tile Q(veh)	1.4	0	-	-	0.5	-	-	0.4



**APPENDIX G:
SYNCHRO REPORTS – 2021 TOTAL AM + PM**

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	120	202	130	170	259	110	205	530	110	80	350	80
Future Volume (vph)	120	202	130	170	259	110	205	530	110	80	350	80
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.99	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.96		1.00	1.00	0.85	1.00	0.97	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1471	1549	1298	1471	1474		2906	2995	1311	1484	2879	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1471	1549	1298	1471	1474		2906	2995	1311	1484	2879	
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	133	224	144	189	288	122	228	589	122	89	389	89
RTOR Reduction (vph)	0	0	115	0	14	0	0	0	83	0	19	0
Lane Group Flow (vph)	133	224	29	189	396	0	228	589	39	89	459	0
Confl. Peds. (#/hr)	1		3	3		1	1		1	1		1
Heavy Vehicles (%)	13%	13%	13%	13%	13%	13%	11%	11%	11%	12%	12%	12%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	11.8	18.8	18.8	14.9	21.9		12.1	29.5	29.5	8.9	26.3	
Effective Green, g (s)	11.8	18.8	18.8	14.9	21.9		12.1	29.5	29.5	8.9	26.3	
Actuated g/C Ratio	0.13	0.20	0.20	0.16	0.24		0.13	0.32	0.32	0.10	0.29	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	188	316	264	237	350		381	959	419	143	822	
v/s Ratio Prot	0.09	0.14		c0.13	c0.27		c0.08	c0.20		0.06	0.16	
v/s Ratio Perm			0.02						0.03			
v/c Ratio	0.71	0.71	0.11	0.80	1.13		0.60	0.61	0.09	0.62	0.56	
Uniform Delay, d1	38.5	34.1	29.8	37.1	35.1		37.7	26.5	21.9	40.0	28.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	11.5	7.2	0.2	16.8	89.0		2.5	1.8	0.2	8.2	1.5	
Delay (s)	50.0	41.3	30.0	54.0	124.1		40.2	28.2	22.1	48.1	29.4	
Level of Service	D	D	C	D	F		D	C	C	D	C	
Approach Delay (s)		40.4			101.9			30.4			32.4	
Approach LOS		D			F			C			C	
Intersection Summary												
HCM 2000 Control Delay			49.2				HCM 2000 Level of Service				D	
HCM 2000 Volume to Capacity ratio			0.84									
Actuated Cycle Length (s)			92.1				Sum of lost time (s)				20.0	
Intersection Capacity Utilization			67.2%				ICU Level of Service				C	
Analysis Period (min)			15									

c Critical Lane Group

Queuing and Blocking Report
Baseline

11/23/2019
























Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	L	T	TR
Maximum Queue (ft)	369	855	345	307	2027	286	364	1581	1566	373	1808	1785
Average Queue (ft)	340	837	10	276	2016	271	360	1477	1405	370	1428	1352
95th Queue (ft)	433	850	127	348	2051	368	371	1842	1828	408	2124	2058
Link Distance (ft)		834			2018			1552	1552		1782	1782
Upstream Blk Time (%)		99			97			85	50		42	29
Queuing Penalty (veh)		0			448			0	0		0	0
Storage Bay Dist (ft)	350		550	300		350	350			380		
Storage Blk Time (%)	84	15	0	60	23	20	100			79		
Queuing Penalty (veh)	275	42	0	212	42	53	265			138		

HCM Signalized Intersection Capacity Analysis

1: OR 99E & Molalla Rd

11/23/2019

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	165	382	300	235	302	90	280	515	150	180	850	125
Future Volume (vph)	165	382	300	235	302	90	280	515	150	180	850	125
Ideal Flow (vphpl)	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750	1750
Total Lost time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		0.97	0.95	1.00	1.00	0.95	
Frpb, ped/bikes	1.00	1.00	0.98	1.00	1.00		1.00	1.00	0.98	1.00	1.00	
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Frt	1.00	1.00	0.85	1.00	0.97		1.00	1.00	0.85	1.00	0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1554	1636	1365	1554	1574		3014	3107	1359	1568	3070	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1554	1636	1365	1554	1574		3014	3107	1359	1568	3070	
Peak-hour factor, PHF	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	172	398	312	245	315	94	292	536	156	188	885	130
RTOR Reduction (vph)	0	0	220	0	8	0	0	0	109	0	9	0
Lane Group Flow (vph)	172	398	93	245	401	0	292	536	47	188	1006	0
Confl. Peds. (#/hr)	1		6	6		1	2		1	1		2
Confl. Bikes (#/hr)												2
Heavy Vehicles (%)	7%	7%	7%	7%	7%	7%	7%	7%	7%	6%	6%	6%
Turn Type	Prot	NA	Perm	Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	3	8		7	4		1	6		5	2	
Permitted Phases			8						6			
Actuated Green, G (s)	18.6	30.5	30.5	23.1	35.0		14.8	37.9	37.9	15.5	38.6	
Effective Green, g (s)	18.6	30.5	30.5	23.1	35.0		14.8	37.9	37.9	15.5	38.6	
Actuated g/C Ratio	0.15	0.24	0.24	0.18	0.28		0.12	0.30	0.30	0.12	0.30	
Clearance Time (s)	4.5	5.5	5.5	4.5	5.5		4.5	5.5	5.5	4.5	5.5	
Vehicle Extension (s)	3.0	3.2	3.2	3.0	3.5		3.0	5.2	5.2	3.0	5.2	
Lane Grp Cap (vph)	227	392	327	282	433		351	927	405	191	933	
v/s Ratio Prot	0.11	c0.24		c0.16	c0.25		0.10	0.17		c0.12	c0.33	
v/s Ratio Perm			0.07						0.03			
v/c Ratio	0.76	1.02	0.28	0.87	0.93		0.83	0.58	0.11	0.98	1.08	
Uniform Delay, d1	52.0	48.2	39.3	50.5	44.7		54.9	37.8	32.4	55.6	44.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	13.5	49.4	0.5	23.5	26.0		15.4	1.5	0.3	60.1	52.9	
Delay (s)	65.5	97.6	39.9	73.9	70.7		70.2	39.2	32.6	115.8	97.1	
Level of Service	E	F	D	E	E		E	D	C	F	F	
Approach Delay (s)		70.9			71.9			47.4			100.0	
Approach LOS		E			E			D			F	
Intersection Summary												
HCM 2000 Control Delay			74.3			HCM 2000 Level of Service			E			
HCM 2000 Volume to Capacity ratio			1.00									
Actuated Cycle Length (s)			127.0			Sum of lost time (s)			20.0			
Intersection Capacity Utilization			92.8%			ICU Level of Service			F			
Analysis Period (min)			15									
c Critical Lane Group												

Queuing and Blocking Report
Baseline

11/23/2019

Intersection: 1: OR 99E & Molalla Rd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	NB	SB	SB
Directions Served	L	T	R	L	TR	L	L	T	T	R	L	T
Maximum Queue (ft)	368	854	460	322	1389	307	363	1578	1549	13	379	1808
Average Queue (ft)	299	834	36	107	341	307	363	1523	1190	1	379	1720
95th Queue (ft)	478	848	255	261	1060	330	368	1708	2097	8	425	2025
Link Distance (ft)		834			2018			1552	1552			1782
Upstream Blk Time (%)		98			2			90	18			84
Queuing Penalty (veh)		0			8			0	0			0
Storage Bay Dist (ft)	350		550	300		350	350			230	380	
Storage Blk Time (%)	69	36	0	0	16		100				80	
Queuing Penalty (veh)	469	168	0	0	38		258				340	

Intersection: 1: OR 99E & Molalla Rd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	1794
Average Queue (ft)	1704
95th Queue (ft)	2042
Link Distance (ft)	1782
Upstream Blk Time (%)	83
Queuing Penalty (veh)	0
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection						
Int Delay, s/veh	1.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	282	30	2	414	50	6
Future Vol, veh/h	282	30	2	414	50	6
Conflicting Peds, #/hr	0	1	1	0	1	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	83	83	83	83	83	83
Heavy Vehicles, %	13	13	16	16	2	2
Mvmt Flow	340	36	2	499	60	7

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	377	0	863 359
Stage 1	-	-	-	-	359 -
Stage 2	-	-	-	-	504 -
Critical Hdwy	-	-	4.26	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.344	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1109	-	325 685
Stage 1	-	-	-	-	707 -
Stage 2	-	-	-	-	607 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1108	-	324 684
Mov Cap-2 Maneuver	-	-	-	-	324 -
Stage 1	-	-	-	-	706 -
Stage 2	-	-	-	-	605 -

Approach	EB	WB	NB
HCM Control Delay, s	0	0	18
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	343	-	-	1108	-
HCM Lane V/C Ratio	0.197	-	-	0.002	-
HCM Control Delay (s)	18	-	-	8.3	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.7	-	-	0	-

HCM 6th TWSC
2: June Way & Molalla Rd

11/23/2019

Intersection						
Int Delay, s/veh	1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	537	55	6	437	35	7
Future Vol, veh/h	537	55	6	437	35	7
Conflicting Peds, #/hr	0	2	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	20	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	89	89	89	89	89	89
Heavy Vehicles, %	10	10	9	9	6	6
Mvmt Flow	603	62	7	491	39	8

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	667	0	1141
Stage 1	-	-	-	-	636
Stage 2	-	-	-	-	505
Critical Hdwy	-	-	4.19	-	6.46
Critical Hdwy Stg 1	-	-	-	-	5.46
Critical Hdwy Stg 2	-	-	-	-	5.46
Follow-up Hdwy	-	-	2.281	-	3.554
Pot Cap-1 Maneuver	-	-	890	-	218
Stage 1	-	-	-	-	520
Stage 2	-	-	-	-	598
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	888	-	216
Mov Cap-2 Maneuver	-	-	-	-	216
Stage 1	-	-	-	-	519
Stage 2	-	-	-	-	593

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	23.9
HCM LOS			C

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	237	-	-	888	-
HCM Lane V/C Ratio	0.199	-	-	0.008	-
HCM Control Delay (s)	23.9	-	-	9.1	-
HCM Lane LOS	C	-	-	A	-
HCM 95th %tile Q(veh)	0.7	-	-	0	-

HCM 6th TWSC
3: Molalla Rd & Access

11/23/2019

Intersection						
Int Delay, s/veh	1.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	18	270	370	2	7	51
Future Vol, veh/h	18	270	370	2	7	51
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	20	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	13	13	13	13	2	2
Mvmt Flow	20	293	402	2	8	55

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	404	0	-	0	736 403
Stage 1	-	-	-	-	403 -
Stage 2	-	-	-	-	333 -
Critical Hdwy	4.23	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.317	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1098	-	-	-	386 647
Stage 1	-	-	-	-	675 -
Stage 2	-	-	-	-	726 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1098	-	-	-	379 647
Mov Cap-2 Maneuver	-	-	-	-	489 -
Stage 1	-	-	-	-	663 -
Stage 2	-	-	-	-	726 -

Approach	EB	WB	SB
HCM Control Delay, s	0.5	0	11.4
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1098	-	-	-	623
HCM Lane V/C Ratio	0.018	-	-	-	0.101
HCM Control Delay (s)	8.3	-	-	-	11.4
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.3

HCM 6th TWSC
3: Molalla Rd & Access

11/23/2019

Intersection						
Int Delay, s/veh	0.9					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	54	490	405	7	5	33
Future Vol, veh/h	54	490	405	7	5	33
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	20	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	10	10	7	7	2	2
Mvmt Flow	59	533	440	8	5	36

Major/Minor	Major1	Major2	Minor2		
Conflicting Flow All	448	0	-	0	1095 444
Stage 1	-	-	-	-	444 -
Stage 2	-	-	-	-	651 -
Critical Hdwy	4.2	-	-	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	2.29	-	-	-	3.518 3.318
Pot Cap-1 Maneuver	1071	-	-	-	236 614
Stage 1	-	-	-	-	646 -
Stage 2	-	-	-	-	519 -
Platoon blocked, %		-	-	-	
Mov Cap-1 Maneuver	1071	-	-	-	223 614
Mov Cap-2 Maneuver	-	-	-	-	357 -
Stage 1	-	-	-	-	610 -
Stage 2	-	-	-	-	519 -

Approach	EB	WB	SB
HCM Control Delay, s	0.8	0	11.9
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1071	-	-	-	561
HCM Lane V/C Ratio	0.055	-	-	-	0.074
HCM Control Delay (s)	8.6	-	-	-	11.9
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.2	-	-	-	0.2

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	2.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	5	249	18	40	336	5	21	5	50	1	5	15
Future Vol, veh/h	5	249	18	40	336	5	21	5	50	1	5	15
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	83	83	83	83	83	83	83	83	83	83	83	83
Heavy Vehicles, %	14	14	14	12	12	12	0	0	0	93	93	93
Mvmt Flow	6	300	22	48	405	6	25	6	60	1	6	18

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	411	0	0	322	0	0	839	830	311	860	838	408
Stage 1	-	-	-	-	-	-	323	323	-	504	504	-
Stage 2	-	-	-	-	-	-	516	507	-	356	334	-
Critical Hdwy	4.24	-	-	4.22	-	-	7.1	6.5	6.2	8.03	7.43	7.13
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	7.03	6.43	-
Follow-up Hdwy	2.326	-	-	2.308	-	-	3.5	4	3.3	4.337	4.837	4.137
Pot Cap-1 Maneuver	1086	-	-	1184	-	-	288	308	734	196	220	486
Stage 1	-	-	-	-	-	-	693	654	-	414	416	-
Stage 2	-	-	-	-	-	-	546	543	-	509	509	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1086	-	-	1184	-	-	262	294	734	171	210	486
Mov Cap-2 Maneuver	-	-	-	-	-	-	262	294	-	171	210	-
Stage 1	-	-	-	-	-	-	689	650	-	412	399	-
Stage 2	-	-	-	-	-	-	497	521	-	460	506	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.2			0.9			14.8			16.2		
HCM LOS							B			C		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	460	1086	-	-	1184	-	-	347
HCM Lane V/C Ratio	0.199	0.006	-	-	0.041	-	-	0.073
HCM Control Delay (s)	14.8	8.3	-	-	8.2	-	-	16.2
HCM Lane LOS	B	A	-	-	A	-	-	C
HCM 95th %tile Q(veh)	0.7	0	-	-	0.1	-	-	0.2

HCM 6th TWSC
4: Cooley Rd & Molalla Rd

11/23/2019

Intersection												
Int Delay, s/veh	3.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↗		↖	↗			↕			↕	
Traffic Vol, veh/h	5	423	67	125	384	1	23	1	60	5	5	5
Future Vol, veh/h	5	423	67	125	384	1	23	1	60	5	5	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	20	-	-	500	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	85	85	85	85	85	85	85	85	85	85	85	85
Heavy Vehicles, %	11	11	11	7	7	7	0	0	0	0	0	0
Mvmt Flow	6	498	79	147	452	1	27	1	71	6	6	6

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	453	0	0	577	0	0	1303	1297	538	1333	1336	453
Stage 1	-	-	-	-	-	-	550	550	-	747	747	-
Stage 2	-	-	-	-	-	-	753	747	-	586	589	-
Critical Hdwy	4.21	-	-	4.17	-	-	7.1	6.5	6.2	7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.1	5.5	-	6.1	5.5	-
Follow-up Hdwy	2.299	-	-	2.263	-	-	3.5	4	3.3	3.5	4	3.3
Pot Cap-1 Maneuver	1062	-	-	972	-	-	139	163	547	132	155	611
Stage 1	-	-	-	-	-	-	523	519	-	408	423	-
Stage 2	-	-	-	-	-	-	405	423	-	500	499	-
Platoon blocked, %	-	-	-	-	-	-	-	-	-	-	-	-
Mov Cap-1 Maneuver	1062	-	-	972	-	-	117	138	547	101	131	611
Mov Cap-2 Maneuver	-	-	-	-	-	-	117	138	-	101	131	-
Stage 1	-	-	-	-	-	-	520	516	-	406	359	-
Stage 2	-	-	-	-	-	-	335	359	-	432	496	-

Approach	EB			WB			NB			SB		
HCM Control Delay, s	0.1			2.3			26.1			31		
HCM LOS							D			D		

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	268	1062	-	-	972	-	-	156
HCM Lane V/C Ratio	0.369	0.006	-	-	0.151	-	-	0.113
HCM Control Delay (s)	26.1	8.4	-	-	9.4	-	-	31
HCM Lane LOS	D	A	-	-	A	-	-	D
HCM 95th %tile Q(veh)	1.6	0	-	-	0.5	-	-	0.4



APPENDIX H: CRASH DATA REPORTS

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

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081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

1 - 3 of 6 Crash records shown.

SER#	P	R	J	S	W	DATE	COUNTY	RD#	FC	CONN#	RD CHAR	INT-TYPE	SPCL USE	A	S	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE								
INVEST	E	A	U	I	C	O	CITY	COMPNT	FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR	QTY	MOVE												
RD DPT	E	L	G	N	H	R	URBAN AREA	MLG	TYP	SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E								
UNLOC?	D	C	S	V	L	K	LONG	MILEPNT	LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC					
03234	N	N	N	N	N	08/26/2015	MARION	1	16		STRGHT	Y	N	CLR	S-1STOP	01	NONE	0	STRGHT								013,004	07		
CITY						WE	WOODBURN	MN	0	PACIFIC HY 99E	NE	(NONE)	UNKNOWN	N	DRY	REAR	PRVTE	NE-SW								000	00			
N						9A	WOODBURN UA	31.68		HILLSBORO-SILV HY	03			N	DAY	INJ	PSNGR CAR		01	DRVR	INJC	22	F	OR-Y		043,026	000	07		
N						45 9 5.56	-122 49 51.59			008100100S00		(04)																		
02858	N	N	N	N	N	07/24/2015	MARION	1	14		INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT									29		
NONE						FR	WOODBURN	MN	0	HILLSBORO-SILV HY	NE	TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW										000	00		
N						UNK	WOODBURN UA	31.70		PACIFIC HY 99E	06	1		N	DAY	PDO	PSNGR CAR		01	DRVR	NONE	51	F	OR-Y		026	000	29		
N						45 9 4.66	-122 49 52.38			008100100S00																				
03475	N	Y	N	N	N	09/11/2015	MARION	1	14		INTER	CROSS	N	CLR	S-1STOP	01	NONE	0	STRGHT									013	07	
CITY						FR	WOODBURN	MN	0	HILLSBORO-SILV HY	NE	TRF SIGNAL	N	DRY	REAR	PRVTE	NE-SW										000	00		
N						8P	WOODBURN UA	31.70		PACIFIC HY 99E	06	1		N	DLIT	PDO	PSNGR CAR		01	DRVR	NONE	67	M	OR-Y		043,026	000	07		
N						45 9 4.66	-122 49 52.38			008100100S00																				
04469	N	N	N	N	N	11/13/2015	MARION	1	14		INTER	CROSS	N	RAIN	ANGL-OTH	01	NONE	0	STRGHT									013	27,04	
CITY						FR	WOODBURN	MN	0	WOODBURN-ESTACADA H	CN	TRF SIGNAL	N	WET	ANGL	PRVTE	E -W										000	00		
N						8P	WOODBURN UA	31.70		PACIFIC HY 99E	02	0		N	DLIT	INJ	PSNGR CAR		01	DRVR	NONE	68	M	OR-Y		016,020	038	27,04		
N						45 9 4.66	-122 49 52.38			008100100S00																				

Disclaimer: The information contained in this report is compiled from individual driver and police crash reports submitted to the Oregon Department of Transportation as required in ORS 811.720. The Crash Analysis and Reporting Unit is committed to providing the highest quality crash data to customers. However, because submittal of crash report forms is the responsibility of the individual driver, the Crash Analysis and Reporting Unit can not guarantee that all qualifying crashes are represented nor can assurances be made that all details pertaining to a single crash are accurate. Note: Legislative changes to DMV's vehicle crash reporting requirement, effective 01/01/2004, may result in fewer property damage only crashes being eligible for inclusion in the Statewide Crash Data File.

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION

TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT

CONTINUOUS SYSTEM CRASH LISTING

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

4 - 6 of 6 Crash records shown.

SER#	P	R	J	S	W	DATE	COUNTY	RD#	FC	CONN#	RD CHAR	INT-TYPE	SPCL USE	TRLR	QTY	MOVE	A	S	ACT	EVENT	CAUSE										
INVEST	E	A	U	I	C	O	CITY	COMPNT	FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED								
RD DPT	E	L	G	N	H	R	URBAN AREA	MLG TYP	SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE		
UNLOC?	D	C	S	V	L	K	LONG	MILEPNT	LRS	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY																	
																01	NONE	0	STRGHT												
01709	N	N	N	N		05/11/2015	MARION	1	14		INTER	CROSS	N	CLR	ANGL-OTH	01	NONE	0	TURN-R												
CITY						MO	WOODBURN	MN	0	WOODBURN-ESTACADA H	CN			DRY	TURN		PRVTE		E	-N											
N						12P	WOODBURN UA	31.70		PACIFIC HY 99E	02	1		DAY	PDO		PSNGR	CAR													
N						45 9 4.66	-122 49 52.38			008100100S00																					
05246	N	N	N	N	N	12/29/2015	MARION	1	14		STRGHT		N	RAIN	S-1STOP	01	NONE	0	STRGHT												
CITY						TU	WOODBURN	MN	0	PACIFIC HY 99E	S	(NONE)	L-GRN-SIG	WET	REAR		PRVTE		S	-N											
N						7A	WOODBURN UA	31.72		WOODBURN-ESTACADA H	05			DAY	PDO		PSNGR	CAR													
N						45 9 3.78	-122 49 53.17			008100100S00		(05)																			

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081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2016 to 12/31/2016, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2016 to 12/31/2016, Both Add and Non-Add mileage

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2017 to 12/31/2017, Both Add and Non-Add mileage

CONTINUOUS SYSTEM CRASH LISTING

081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2017 to 12/31/2017, Both Add and Non-Add mileage

6 - 7 of 7 Crash records shown.

SER#	P	R	J	S	W	DATE	COUNTY	RD#	FC	CONN#	RD CHAR	INT-TYPE	SPCL USE	TRLR	QTY	MOVE	A	S	ACT	EVENT	CAUSE										
INVEST	E	A	U	I	C	DAY	CITY	COMPNT	FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED								
RD DPT	E	L	G	N	H	R	URBAN AREA	MLG	TYP	SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	TO	P#	TYPE	SVRITY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE			
UNLOC?	D	C	S	V	L	K	LONG	MILEPNT	LRS	(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRITY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE			
05532	N	N	N	N	N	N	MARION	1	14		INTER	CROSS	N	CLD	O-1 L-TURN	01	NONE	0	TURN-L										04		
CITY						TH	WOODBURN	MN	0	WOODBURN-ESTACADA	H	CN	TRF	SIGNAL	N	DRY	TURN	PRVTE	N	-E							022	00			
N						5P	WOODBURN UA	31.70	PACIFIC HY 99E	04	0		N	DLIT	INJ		PSNGR	CAR		01	DRVR	NONE	42	F	OR-Y	020	022	04			
N						45 9 4.66	-122 49 52.38			008100100S00																					
													02	NONE	0	STRGHT												000	00		
																											000	000	00		
													03	NONE	0	STRGHT												022	00		
																											000	022	00		
05491	N	N	N	N	N	N	MARION	1	14		STRGHT		Y	CLR	S-1STOP	01	NONE	0	STRGHT									013	07		
CITY						WE	WOODBURN	MN	0	PACIFIC HY 99E	S	(NONE)	L-GRN-SIG	N	DRY	REAR	PRVTE	S	-N								000	00			
N						10A	WOODBURN UA	31.72	WOODBURN-ESTACADA	H	05		N	DAY	INJ		PSNGR	CAR		01	DRVR	NONE	19	M	OR-Y	043,026	000	07			
N						45 9 3.78	-122 49 53.17			008100100S00		(05)																			
													02	NONE	0	STOP													012	013	00
																												000	022	00	
													02	NONE	0	STOP													012	013	00
																												000	000	00	
													02	NONE	0	STOP													012	013	00
																												000	000	00	
													03	NONE	0	STOP													012	00	
																												000	000	00	

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081: PACIFIC HIGHWAY EAST

Highway 081 ALL ROAD TYPES, MP 31.65 to 31.74 01/01/2017 to 12/31/2017, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

1 - 4 of 4 Crash records shown.

SER#	P	R	J	S	W	DATE	COUNTY	RD#	FC	CONN#	RD CHAR	INT-TYPE	SPCL USE	A	S															
INVEST	E	A	U	I	C	DAY	CITY	COMPNT	FIRST STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	TRLR QTY	MOVE	PRTC	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE			
RD DPT	E	L	G	N	H	R	URBAN AREA	MLG	TYP	SECOND STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	P#	TYPE	SVRTY	E	X	RES	LOC					
UNLOC?	D	C	S	V	L	K	LONG	MILEPNT	LRS			(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO											
00054	N	N	N	N		01/07/2014	MARION	1	14		STRGHT	Y		N	RAIN	S-1STOP	01	NONE	0		STRGHT									
NONE						TU	WOODBURN	MN	0	HILLSBORO-SILV HY	W	(NONE)	UNKNOWN	N	WET	REAR		PRVTE			W	-E				000	00			
N						3P	WOODBURN UA	39.25		PACIFIC HY 99E	03			N	DAY	INJ		PSNGR CAR			01	DRVR	INJC	55	M	OR-Y	026	000	07	
N						45 9 4.738176	-122 49 55.416684			014000100S00		(03)																		
																	02	NONE	0		STOP						011	00		
																		PRVTE			W	-E				000	000	00		
																		PSNGR CAR			01	DRVR	NONE	50	M	OR-Y	000	000	00	
02111	N	N	N	N	N	06/27/2014	MARION	1	14		STRGHT	N		N	CLD	O-STRGHT	01	NONE	0		STRGHT									
CITY						FR	WOODBURN	MN	0	PACIFIC HY 99E	W	(NONE)	NONE	N	DRY	HEAD		PRVTE			E	-W				000	00			
Y						4P	WOODBURN UA	39.25		HILLSBORO-SILV HY	03			N	DAY	PDO		PSNGR CAR			01	DRVR	NONE	39	M	OR-Y	039,080	000	05	
N						45 9 4.738176	-122 49 55.416684			014000100S00		(04)																		
																	02	NONE	0		STRGHT							000	00	
																		PRVTE			W	-E				000	000	00		
																		PSNGR CAR			01	DRVR	NONE	45	F	OR-Y	000	000	00	
04710	N	N	N	N	N	12/29/2014	MARION	1	14		STRGHT	N		N	CLR	S-1STOP	01	NONE	0		STRGHT									
CITY						MO	WOODBURN	MN	0	HILLSBORO-SILV HY	W	(NONE)	UNKNOWN	N	DRY	REAR		PRVTE			W	-E				000	013	07		
N						5P	WOODBURN UA	39.25		PACIFIC HY 99E	03			N	DUSK	PDO		PSNGR CAR			01	DRVR	NONE	65	M	OR-Y	026	000	07	
N						45 9 4.74	-122 49 55.42			014000100S00		(03)																		
																	02	NONE	0		STOP							011	013	00
																		PRVTE			W	-E				000	000	00		
																		PSNGR CAR			01	DRVR	NONE	39	F	OR-Y	000	000	00	
02826	Y	N	N	N	N	08/21/2014	MARION	1	14		STRGHT	Y		N	CLR	S-1STOP	01	NONE	0		STRGHT									
CITY						TH	WOODBURN	MN	0	HILLSBORO-SILV HY	W	(NONE)	UNKNOWN	N	DRY	REAR		PRVTE			W	-E				000	00			
N						9P	WOODBURN UA	39.27		PACIFIC HY 99E	03			N	DLIT	INJ		PSNGR CAR			01	DRVR	NONE	28	M	NONE	047,026	000	01,07	
N						45 9 4.6995119	-122 49 53.8969079			014000100S00		(03)																		
																	02	NONE	0		STOP							011	00	
																		PRVTE			W	-E				000	000	00		
																		PSNGR CAR			01	DRVR	INJC	16	M	OR-Y	000	000	00	

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140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2016 to 12/31/2016, Both Add and Non-Add mileage

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Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2016 to 12/31/2016, Both Add and Non-Add mileage

140: HILLSBORO-SILVERTON

Highway 140 ALL ROAD TYPES, MP 39.24 to 39.29 01/01/2017 to 12/31/2017, Both Add and Non-Add mileage

161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2013 to 12/31/2013, Both Add and Non-Add mileage

161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

OREGON DEPARTMENT OF TRANSPORTATION - TRANSPORTATION DEVELOPMENT DIVISION
TRANSPORTATION DATA SECTION - CRASH ANALYSIS AND REPORTING UNIT
CONTINUOUS SYSTEM CRASH LISTING

161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

5 - 5 of 5 Crash records shown.

SER#	P	R	J	S	W	DATE	COUNTY	RD#	FC	CONN#	RD CHAR	INT-TYPE	SPCL USE	TRLR	QTY	MOVE	A	S	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE			
INVEST	E	A	U	I	C	O	CITY	COMPNT	FIRST	STREET	DIRECT	(MEDIAN)	INT-REL	OFFRD	WTHR	CRASH	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE		
RD DPT	E	L	G	N	H	R	URBAN AREA	MLG TYP	SECOND	STREET	LOCTN	LEGS	TRAF-	RNDBT	SURF	COLL	OWNER	FROM	PRTC	INJ	G	E	LICNS	PED	ERROR	ACT	EVENT	CAUSE		
UNLOC?	D	C	S	V	L	K	LONG	MILEPNT	LRS		(#LANES)	CONTL	DRVWY	LIGHT	SVRTY	V#	TYPE	TO	P#	TYPE	SVRTY	E	X	RES	LOC	ERROR	ACT	EVENT	CAUSE	
														01	NONE	0	STRGHT													
																	PRVTE	E -W											000	00
																	PSNGR	CAR	02	PSNG	INJC	30	M					000	000	00
														01	NONE	0	STRGHT													
																	PRVTE	E -W											000	00
																	PSNGR	CAR	03	PSNG	INJB	55	M					000	000	00
														01	NONE	0	STRGHT													
																	PRVTE	E -W											000	00
																	PSNGR	CAR	04	PSNG	INJA	20	M					000	000	00
														02	NONE	0	STRGHT													
																	PRVTE	W -E											000	00
																	PSNGR	CAR	01	DRVR	INJA	57	M	OR-Y				000	000	00

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161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2014 to 12/31/2014, Both Add and Non-Add mileage

161: WOODBURN-ESTACADA

Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

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Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2015 to 12/31/2015, Both Add and Non-Add mileage

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Highway 161 ALL ROAD TYPES, MP 0.00 to 0.44 01/01/2017 to 12/31/2017, Both Add and Non-Add mileage